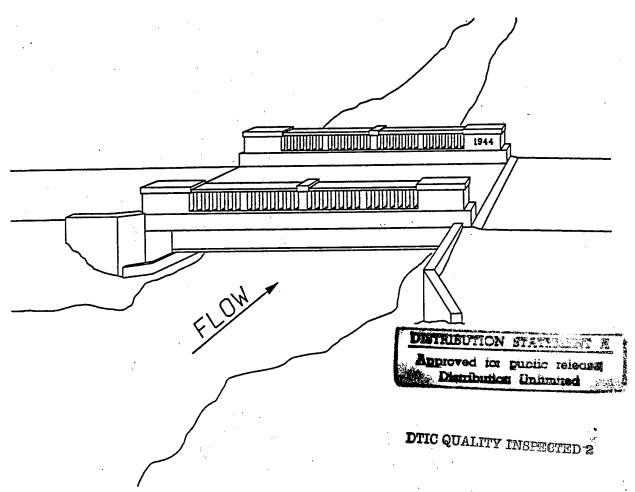


Bridge Inspection Program

FY 93 Routine Inspections



October 1993

New England Division

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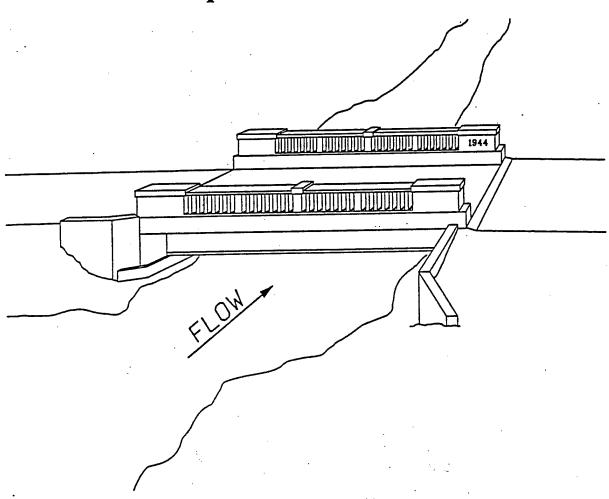
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Bridge Inspection Program

FY 93 Routine Inspections



October 1993

New England Division

BRIDGE INSPECTION PROGRAM FY 93 ROUTINE INSPECTIONS NEW ENGLAND DIVISION

OCTOBER 1993

DEPARTMENT OF THE ARMY
NEW ENGLAND DIVISION, CORPS OF ENGINEERS
WALTHAM, MASSACHUSETTS

TABLE OF CONTENTS

<u>Project</u>	<u>Bridge</u>	<u>Page</u>
Franklin Falls, N.H.	Lower Mill Brook	6
11 11 11	Upper Mill Brook	. 11
11 11 11	Knox Brook	16
11 11 11	Blake Brook	20
11 11 11	Smith River	24
Birch Hill, MA	Middle Road	29
11 11	New Boston Road	33
11 11	Burgess Road	37
11 11	Old Route 202	41
11 11	Goodnow Road	45
West Hill, MA	West Hill Road	49
Thomaston, CT	Leadmine Brook Road	54
Northfield Brook, CT	Old Route 254 (Upper)	58
n n	" " (Lower)	62
Black Rock, CT	Old Northfield Road	66
Hop Brook, CT	Old Route 63	70
Tully Lake, MA	Doane Hill Road	74
Everett Lake, NH	Choate Brook	79
Otter Brook, NH	Rec Area (Exit)	83
11 11	Rec Area (Entrance)	111
Colebrook, CT	Old Route 8 (Bridge in MA)	116
Knightville, MA	Indian Hollow	121

TABLE OF CONTENTS

Appendix A Visual Assessment for Scour Potential A1

Everett Lake --- Choate Brook

Birch Hill ---- Goodnow Road Old Route 202

Middle Road

NEW ENGLAND DIVISION FY 93 ROUTINE BRIDGE INSPECTION PROGRAM

PURPOSE AND SCOPE

The purpose of the routine bridge inspections is to inspect the physical condition of the structures and to verify and update the findings and evaluations reported in the last in-depth and routine inspection. All previously detected areas of structural distress or operational inadequacies were reevaluated and any new deficiencies documented with the overall goal being to increase the useful life of the structures and to ensure the continued safety of the bridge users.

AUTHORITY

The basis for the inspections is contained in ER 1110-2-111 "Periodic Safety Inspection and Continuing Evaluation of United States Army Corps of Engineers Bridges."

INSPECTION PROCEDURE

The overall inspections were performed in accordance with AASHTO's 1983 "Manual for Maintenance Inspection of Bridges", the Department of Transportation's "Bridge Inspector's Training Manual 90" (1990 edition) and all applicable sections of ER 1110-2-111. The inspection program was carried out under the direct supervision of a licensed Professional Engineer. The most recent in-depth inspection reports were thoroughly reviewed by inspection personnel prior to and during the field inspections.

The underside of all smaller Reservoir Area bridges were accessed using a ladder, waders and a small boat, or some combination thereof, as required.

During all inspections, all pertinent safety equipment was utilized and all pertinent safety procedures were followed.

REPORTING PROCEDURE

For each bridge, an overall summary has been prepared. Included are the vehicle ratings, evaluation of each structural component, and overall structural evaluation, all compared with those from all previous inspections. Also included are the previous recommended remedial repairs, the status of these recommendations and any new recommendations and/or comments based on the current inspections.

Field-completed checklists for each bridge are the Standard Structures Inspection Field Report and the Scour Checklist (an NED devised checklist based on recent Federal Highway Administration guidelines to more precisely address any potential or active scour-related problems).

BRIDGES INSPECTED

For the 1993 Interim Bridge Inspection Program, 22 bridges were inspected as indicated herein. Bridges inspected, projects, 1993 and 1991 condition ratings, inspection dates, estimated rehabilitation costs, rehabilitation priorities (see below) with temporary posting required, and degree of existing scour (see below) are summarized on next page:

Rehab. Priority (Posting, if necessary, in tons required)

- Bridge currently cannot tolerate present traffic/loads.
 Prompt remedial measures are required. Bridge should be posted and restricted as indicated until corrective measures can be accomplished.
 - 2. Major items require rehabilitation. Minimum adequacy to tolerate present traffic/loads. Further deterioration may cause priority 1.
 - 3. Minor items require rehabilitation to maintain condition.

Scour

- 1 Major Scour Activity/Potential
- 2 Moderate Scour Activity/Potential
- 3 Minimal or No Scour Activity/Potential

Project/Bridge Rehab. Scour		Condition		Date		Est.Rehab.		b.
		Rating		Ins	pected	Cost	(K)	
Priority		1991 1993	3					(see
below)		1001 100.						(555
FRANKLIN FALLS								
1. Lower Mill Brook	4		7/14		55.5		1*	3**
Upper Mill Brook	4	4	7/15				1*	2**
3. Knox Brook	4	9			0.0		3	2
4. Blake Brook	7	7			15.0		3	3
5. Smith River	5	5	7/14	/93	180 <u>+</u>		1	(5) *2**
BIRCH HILL							_	
Middle Road	7	7	6/25				3	3**
New Boston Road	7	8	6/24				3	3
8. Burgess Road	7	7	6/24				2	3
9. Old Route 202	7	7	6/24				3	2**
10. Goodnow Road	7	7	6/25	/93	5.0		3	2**
WEST HILL								
11. West Hill Road	5	7	9/08	/93	91.5		2	2
THOMASTON								_
12. Leadmine Brook	8	8	8/24	/93	0.0		3	2
NORTHFIELD BROOK								
13. Old Rt.254 (upper)		7	8/24	•	0.0		3	2
14. Old Rt.254 (lower)	8	7	8/24	/93	0.0		3	3
BLACK ROCK		_					_	_
15. Old Northfield Rd.	8	8	8/24	/93	31.0		3	3
HOP BROOK		_					_	_
16. Old Route 63	5	. 7	8/24	/93	0.5		3	3
TULLY LAKE	_	<u>_</u>					_	•
17. Doane Hill Road	7	, 7	6/25	/93	25.0		3	3
EVERETT LAKE		_					_	0.1.1.
18. Choate Brook	4	7	9/09	/93	0.0		3	2**
OTTER BROOK	_	_					_	
19. Rec Area (Exit)	. 6	6		/93			2	2**
20. Rec Area (Entran.)	7	7	8/18	/93	32.0		2	2**
COLEBROOK	_	_	0 /0-		0 0		_	
21. Old Route 8	7	8	8/25	/93	0.0		3	3
KNIGHTVILLE	_	•	0 /05	102	5 0		2	3
22. Indian Hollow	7	7	8/25	1/93	5.0		3	3

LEGEND

See overall assessment.
Scour analysis performed. **

OVERALL ASSESSMENT

During FY93, only reservoir area bridges (no spillway bridges) were inspected. Overall, the condition of the bridges inspected ranged from good to fair to poor, with overall condition ratings and rehabilitation priorities as listed above.

REHAB PRIORITY 1

Bridges that were assessed a rehab priority of 1, with corresponding reduction in capacity are as follows:

<u>Project</u>	<u>Bridge</u>	<u>Temporary Posting</u>
Franklin Falls	Upper Mill Brook	close
Franklin Falls	Lower Mill Brook	6
Franklin Falls	Smith River	5

These three bridges have been given the highest priority, with recommendations listed herein to be expeditiously carried out. Until these bridges have been rehabilitated as indicated, the above posting for each bridge shall be strictly adhered to.

Operation Directorate has made an assessment of the future intended usage of the bridges to determine what level of rehabilitation, if any, is required. Based on their decision, the following has been recommended by Engineering Directorate with concurrence from Operations Directorate:

- 1. Upper Mill Brook will be permanently closed to vehicular traffic by installation of permanent barriers on the east and west approaches.
- 2. Lower Mill Brook will be rehabilitated as recommended with design and construction budgeted for FY 94.
- 3. Smith River Bridge will be immediately posted for a 5 tons weight rating and 10 mph speed limit in order to limit usage to small truck traffic. Interim inspections will be performed on the structure at six month intervals to determine if further deterioration requires further reduction of capacity or complete closure.

FRACTURE CRITICALITY

Of the bridges inspected, only Old Route 8 Bridge falls into the fracture critical category. It is a two truss, simple span, through truss, steel structure with built up members and riveted connections. Some of the rivets have been replaced with high strength bolts. Because of its location (within the reservoir), it is subjected to very minimal traffic, in general, and therefore, minimal truck traffic. Because of this low traffic volume and the overall good condition of the structure, no additional testing is required and continued two year inspection

intervals is considered sufficient.

SCOUR

The FY 93 routine inspections also include a scour checklist (an NED devised checklist based on recent FHWA guidelines) which was encorporated to better define any active or potential scour related problems.

Scour problems have been noted at the following bridges and listed in order of relative severity:

Otter Brook ---- Recreation Area (Exit)
Otter Brook ---- Recreation Area (Entrance)
Birch Hill ---- Goodnow Road
Thomaston ---- Leadmine Brook
Birch Hill ---- Old Route 202
Everett Lake --- Choate Brook

Remedial measures have been listed in the recommendation section of the text for each structure. None of these conditions are considered to be of such criticality that immediate action is warranted. Repairs should be performed in a timely manner through normal budgetary procedures and priorities, and continued monitoring of scour conditions should be performed during all future inspections.

FY 93 scour assessments, both Hydrologic/Hydraulic and Geotechnical were performed on bridges at Everett Lake (Choate Brook) and Birch Hill (Goodnow Road, Middle Road, Old Route 202) with the results encorporated into the inspection reports. This makes a total of nine bridges as indicated in the summary above, in the inventory, which have been assessed in this manner for scour criticality. Based on the extremely low probability of failure from scour, it is recommended that not further in-depth Hydrologic/Hydraulic scour assessments be performed, on any bridges, unless recommended as a result of specific findings during future inspections.

FRANKLIN FALLS DAM LOWER MILL BROOK BRIDGE, HILL, NH FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 14 July 93

<u>DATE OF PREVIOUS INSPECTIONS</u>: In-Depth, 9, 10 July 85

Routine, 17 July 87 Routine, 28 April 89 Routine, 15 May 91

RATING (T=TONS)

Type Inventory Operating Comments

H15 6.8T 15.2T No change in ratings

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway and Railings: Overall condition is poor (condition 4).

The gravel wearing surface is well

graded. Vegetative growth and debris are evident on the deck and approaches. The growth includes one 4- and one 6- inch tree on the shoulder. The approaches are in good condition. The transitions are good on both approaches. The load rating is not posted. There are no drains on the deck. The steel pipe bridge and approach railings are missing large sections and are heavily

the streambed.

B. Fascias & Curbs: The fascias and curbs are in fair

condition. There is some concrete spalling evident on the exterior of the west fascia. There is minor

corroded. There is a steep drop to

efflorescence from the concrete over and

around the exterior steel beams.

C. Underside of Deck: The overall condition of the underside

of the concrete deck is fair (condition

5). One moderate spall was noted on the

inside southwest corner of the

exterior concrete arch beam. The area of this spall was described in the 91

investigation to be approximately four

square feet. There has been no

significant increase in size since that observation. Minor efflorescence was observed on the underside of the deck.

Minor cracks were observed in the

concrete arch between the two exterior beams. There is some minor surface corrosion of the exposed steel beams and tie rods. Graffiti is evident.

D. Wingwalls/Abutments:

The condition of the abutments is fair (condition 5). The north and south abutments contain minor hairline cracks and efflorescence. There is an approximately 36-inch crack at the junction of the north abutment and west bridge deck which intersects the west wingwall.

The overall condition of the channel training walls is fair (condition 5). The northwest channel training wall has minor cracks. The southwest channel training wall is of stone rubble masonry and exhibits minor effects of erosion. The mortar is eroded but there is no evidence of rock loss or movement.

The wingwalls are is fair condition. All wingwalls have minor cracks, spalls and efflorescence. Wingwall drainage pipes are covered by vegetation. No catch basin was observed. Several full-length vertical cracks were found on northeast and northwest wingwalls.

E. Channel:

The channel shows no sign of scour. There are no obstructions or debris in the channel.

CONDITION RATINGS:

repair sidewalk.

In-depth, 1985: 7
Routine, 1989: 5
Routine, 1991: 4
Routine, 1993: 4

Status of Previous Recommendations

Item Current Status

1. Install steel beam guardrail. Not Done

2. Repair cracks at approaches and NW corner of deck.

3. Patch spalled concrete and Not Done

- 4. Sandblast and paint steel beams. Not Done
- 5. Regrade roadbed. Done

Revised Recommendations

1. Install steel beam guardrail.

Estimated cost \$30,000.

2. Repair cracks on the wingwalls of the north abutments and crack at NW corner of the deck. Patch mortar on the southwest training wall.

Estimated cost \$5,000.

3. Patch all areas of spalled concrete on fascia and curbs and the inside southwest corner of the exterior arch beam. Repair sidewalk.

Estimated cost \$10,000.

- Sandblast and paint exposed steel surfaces.
 Estimated cost \$10,000.
- 5. Post load rating on approaches.

Estimated cost \$500.

Total cost \$55,500.

				9						
		STRUC	TURES IN ROUT	I SPECT TINE INS				rankl ower.	in FallEDa Mill Brook	m Bridg
Ci., . (H:11	NH		bridge dep	t. no.	8-structu	re no. EDN 4 33 1000	7 4	90-date inspo 14 July	
2-dist.	104-highway sy		22-owner	<u> </u> E	27-year	built	106-year rebuilt		11-milepoint	
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PROJECT: Franklin Falls Dam
NAME: Lower Mill Brook Bridge
LOCATION: Hill NH

BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	425
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	yes
a. Piers, abutments with spread footings or short pile foundations.	425
b. Superstructure with simple spans or non- redundant support systems.	428
c. Inadequate waterway openings.	<u></u>
d. Designs which collect ice and debris.	no
e. All water must pass through or over structure.f. Other.	y 28
3. Are any characteristics of an aggressive stream or waterway present?	no·
a. Active degradation or aggredation of streambed.	no
b. Significant lateral movement or erosion of streambanks.	no
c. Steep slopes.	no
d. High velocities.	no
e. Any history of highway or bridge damage during past floods.	no
f. Other.	
4. Is the bridge located on a stream reach with any	no
adverse flow characteristics?	110
a. Crossing near stream confluence.	Vo
 b. Crossing of tributary stream near confluence with larger streams. 	00
c. Crossing on sharp bend in stream.	No
d. Location on alluvial fan. e. Other.	no —
5. Other comments or observations.	yes
Minor erosion (mortar scour) on southwest channel traini	ng wall.

FRANKLIN FALLS DAM UPPER MILL BROOK BRIDGE, HILL, NH FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 15 July 93

<u>DATE OF PREVIOUS INSPECTIONS</u>: In-Depth, 9, 10 April 85

Routine, 17 July 87 Routine, 28 April 89

13 May 91

Routine,

RATING (T=TONS)

Type Inventory Operating Comments

H15 7.2T 10.7T No change in ratings

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway and Railings: Overall condition is poor (condition 4).

The gravel wearing surface is very rutted. There are deep (3 inch) depressions evident at several spots on the deck. The depth of gravel wearing surface on the concrete deck is indeterminate and it is not possible to determine whether the top of the deck is damaged. Vegetative growth and debris are evident on the deck and approaches. A 10-inch tree grows on the approach near the southwestern retaining wall. There are no drains on the deck. The steel pipe bridge and approach railings are heavily rusted with large sections missing. There is a steep drop to the streambed.

The eastern approach is in poor condition with excessive settlement on the north edge of the road where the embankment and approach retaining wall are slumping. Three Jersey barriers have been placed along the north edge of the road to steer vehicles away from the failing embankment. There is a sharp transition at the east approach and a steep drop to the streambed below. The western approach is similarly rutted but the transition is good. The load rating is not posted at either approach.

B. Fascias & Curbs: The fascia is in fair condition

B. Fascias & Curbs:

The fascia is in fair condition (condition 5). The curbs are hidden by a thick gravel wearing surface. There is moldy growth along the exterior edge of the steel beams.

C. Underside of Deck:

The overall condition of the underside of the concrete deck is good (condition 7). There are no spalls or cracks, but there is some efflorescence. There is some minor surface corrosion of the exposed steel beams and tie rods.

D. Wingwalls/Abutments:

The condition of both abutments is good (condition 7). The east and west abutments contain cold joints.

Minor efflorescence is evident. The western abutment has 24 to 28 inches of moderate scour under an apron of the same width. The eastern abutment is slightly undermined to a depth of 16 inches under a solid apron of approximately 3 feet.

The southwest wingwall has minor cracks and efflorescence. The northwest wingwall is very overgrown by trees and other vegetation. Its general appearance is the same as that of the southwest wall. There is a 5 foot (full length), 1/2 inch wide vertical crack halfway along the wall. The southeast wingwall is covered with vegetative growth. There is some minor There is a full length, efflorescence. full depth (3 inch), 1-inch wide vertical crack midway along the wall. The northeast wingwall embankment is undercut by scour. There is a full length, full depth (3 inch), 1 1/4 inch There is a wide crack at the 1/3 point. full length, full depth, 2-inch wide crack halfway along the wall. The FY 91 bridge inspection contains diagrams detailing the site. The horizontal distance along crack 2 was measured to be 5 7/8 inches in the 91 report vs 5 3/4 inches in 93. A full length, full depth, 4-inch wide crack is located at the 2/3 point along the wall. The wall has rotated outward from the bank.

E. Channel:

There is an 8-inch diameter corroded cast iron pipe crossing the upstream side of the streambed. There are numerous boulders and cobbles throughout the channel. A bend in the streambed downstream of the bridge is causing eddies which are undermining the east embankment.

CONDITION RATINGS:

In-depth, 1985: 7
Routine, 1989: 5
Routine, 1991: 4
Routine, 1993: 4

Status of Previous Recommendations

Item

Current Status

1. Temporarily close bridge and extend barriers at north and south ends.

Not Done

2. Complete scour analysis.

Done

Revised Recommendations

The hydrologic and hydraulic assessment of Upper Mill Brook Bridge completed in 1992 recognizes a need to repair the stone and mortar aprons surrounding the bridge abutments. It also recommends that a 100 foot stone revetment which would vary in height from 5 to 15 feet be placed along the steambed to control bank erosion. The revetment would consist of 2 to 3 feet of stone protection underlain by 1 to 1.5 feet of stone bedding. Granular fill will be needed to fill eroded areas behind the revetment. A small stone dam downstream from the bridge which could be altering the direction of stream flow may need to be removed. This would require rental of a crane for a few hours to remove the stone.

Total cost \$40,000

The report notes that conditions at the bridge are severe and the cost of repairs high. It recommends closure of the bridge by installing permanent barriers on east and west approaches allowing only pedestrian and bicycle traffic. (A park gate presently exists on the east approach road only.)

Total cost (40 feet of Jersey barrier) \$500

STRUCTURES INSPECTION FIELD REPORT Frank Un Falls Dam

BOUTINE INSPECTION

Franklin Falls Dam Upper Hill Breck Bridge

JA-INACCESSIBLE

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PROJECT: Franklin Falls Dam

NAME: Upper Mill Brook Bridge

LOCATION: Hill NH

BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	y 25
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	ye5
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	425 425 425 425
3. Are any characteristics of an aggressive stream or waterway present?	<u>yes</u>
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	yes yes des yes
4. Is the bridge located on a stream reach with any adverse flow characteristics?	૧૦
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	no no no
5. Other comments or observations.	yes
Erosion beneath northeast embankment causing slump.	

FRANKLIN FALLS DAM KNOX BROOK BRIDGE FISCAL YEAR 1993 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION:

14 July 93

DATE OF PREVIOUS INSPECTIONS:

Routine Inspection, 14 May 91 Inventory Inspection, April 85

RATING (T = TONS)

Type	Inventory	Operating	Comments
H	20 T Esti	mated	The bridge was reconstructed in 1992. Load rating calculations are forthcoming.

EVALUATION (See attached "Structures Inspection Field Report")

A. Superstructure
-Above Deck

- -Overall condition is very good.
- -The bridge was rebuilt in 1991. The stone abutments were capped with new concrete bridge seats. The superstructure is constructed of prestressed concrete planks.
- -Both north and south approaches are in fair condition. The gravel roadway is slightly rutted as it transitions to the bridge deck.
- -The southeast stone wingwall is capped with three W12 steel beams, presumably salvaged from the old superstructure.
- -The joint at the interface between the south west stone wingwall and the new concrete abutment is wide and allows gravel to wash down off of the road.
- -There are no approach guard rails or bridge railings.
- -Joints between the deck and both abutment backwalls are improperly sealed with concrete. This is causing some cracking and spalling at the joint.
- -All of the wingwalls are in good condition, with only moderate growth of vegetation between the stones.

В.	Superst	ructure
	-Below	Deck

-Overall condition is very good.

-There is a foam backer rod protruding from between the two eastern precast planks.

-Underside of deck is in good condition.

C. Substructure

-Overall condition is good.

-The stone abutments are in good condition.

-There is a beaver dam constructed against the upstream (east) wingwalls.

-Clear water is flowing out from between the stones of both abutments. The water is flowing from behind the beaver dam. The water does not appear to be carrying soils out from behind the

abutments.

-Slight scour is present under the north abutment.

D. Channel

-The channel under the bridge is in fair condition, with only slight scouring.

E. Overall Numerical Condition Rating

Inventory 1985: 7
Routine 1991: 4
Routine 1993: 9

RECOMMENDATIONS

Status of Previous Recommendations

- Post the load limit at both approaches.
- 2. Remove existing deck and girders, and recap both abutments with new concrete bridge seats. Install a new prestressed concrete plank bridge deck with parapets.
- Install guard rails on both approaches.

Item No. 2 has been completed. Items 1 and 3 have not.

Revised Recommendations

Post the load limit at both approaches.

STRUCTURES INSPECTION FIELD REPORT

ROUTINE INSPECTION

city/t				bridge dept	. no.	8-struct		90-date	e inspected
	an Bornto			1			EDNH3310006	7/	4/93
2-dist.	104-highway sy	/ /	22-owner <i>CO</i>	<u>-</u>	27-year	built 7 <i>20</i>	106-year rebuilt	11-mile	point
43-structur	re type		_		quality o	ontrol en			
Prest	tressed, Pre	cast Concre	ete Plan	45		Nich	r. Forbes		
07-facility		,			team le	ader Joe.	Colveci		
	s intersected	f				nembers <i>alsh</i>	M. Deschenes , L	L. Bra	chaner
item 58	3	9	item 59			9	item 60		8
DECK			SUPERS	TRUCTURE		لــنّــا	SUBSTRUCTURE	Ē	L 12
	earing Surface	9	1. Bearin	g Devices		9	1. Abutments		-
	•	9	2. String	ers		N	a-Wings b-Backw	oli.	7
	eck-Condition		3. Diaph	ragms		N	c-Bridge		9
	ay in Place Forms	9	4. Girder	s or Beams	Planks	9	d-Breast		7
4. Cu	urbs		5. Floor			N	e-Footing	gs	7
5. Me	edian	N	6. Trusse	es		N	f-Piles		\mathcal{N}
6. Sid	dewalks	N	7. Rivets	or Bolts		N	g-Erosio	n	7 .v
7. Pa	arapet	N	8. Welds	.		N	h-Settler	nent	N
8. Ra	ailina	N	9. Collisi	on Damage		N	2. Piers or Bents		
	nti Missile Fence	N	10. Load	-		9	a-Caps b-Colum	n	1
10. Dr		N		er Alignment		9	c-Web	11	N
		N	12. Load	_		N	d-Footin	a	N
	ghting Standards	[AZ]	13. Paint-			N	e-Piles		N
12. Ut	tilities	7	14. Year I			N	f-Scour		N
13. De	eck Joints	ــــا		Clearance _	ft	in	g-Settler		N
14. Ap	pproach Settlemer	nt 7	Clearand		\		3. Collision Dama	-	
			Clearan	Se Olgris		.5 110	4. Hydraulic-Adeo	quacy	8
Actual Po	ostina	H 3 3S2	Single		Overhe	_	(attached to bridge)		
/ total i t	ooung					yes	X no		
Recomm	nended Posting				1. Wel	de	N		
From Ra	ting Book				1. ***	J			
					2. Bolt	5			
SIGNS I Y or N	IN PLACE	at bridge	advar	nce	3. Con	dition	N		
LEGIBIL	_ITY	-			Item93	b U/W I	Inspection Date: MA		8 T
	04 -1- ' '				1		f. A. f. A.		
I IEM	61-channel and	channel protection	<u> </u>		36-T	гапіс Sa	fety features 36	con	dition
1.0118	annel scour	7 5. rip r	ap or slope p	aving 🕢	1. br	idge raili			
+	bankment erosio	on 7 6. effe	ctiveness	7	2. tra	ansitions	8		
	der system	7. deb		7			guardrail N		
4. spu	ur dikes & jetties	ריי∟ צ. veg	etation	<u></u>	4. gt	uardrail te	erminai	ı L	

SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	_ <u>/es</u> _
2. Is streambed erodible? If so, does the structure have any vulnerable design features?	<u>Yes_</u>
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructures with simple spans or non-redundant support systems. c. Inadequate waterway opening. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	Stone Abutment
 3. Are any characteristics of an aggressive stream or waterway present? a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	N
4. Is bridge located on stream reach with any adverse flow characteristics?	
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	
5. Other comments or observations.	

FRANKLIN FALLS DAM BLAKE BROOK BRIDGE, NEW HAMPTON, NH FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 14 July 93

DATE OF PREVIOUS INSPECTIONS: Inventory, April 85

Routine, 16 July 87 Routine, 30 May 89 Routine, 14 May 91

RATING (T=TONS)

Type	Inventory	Operating	Comments
H10	14.5T	19.4T	Load capacity has not changed since previous inspection.

EVALUATION (See attached "Structures Inspection Field Report")

A. Superstructure
Above Deck

The overall condition of the superstructure is good. (condition) 7 There is some minor rutting at each of the gravel approaches. The new timber deck is in very good condition. Sand is accumulating along the brush blocks on either side of the bridge and is preventing adequate drainage of the bridge deck. The 15 ton rating signs at each bridge approach have been vandalized with graffiti and are illegible. The guardrails are in good condition. One post at the north end of the west guardrail is loose. One bolt is missing on the west guardrail at the third support from the south approach. Vegetation is encroaching upon each approach.

B. Superstructure Under Deck

The overall condition of the substructure is good (condition 7). There is minor to moderate rusting of all structural steel. The existing paint system is in poor condition. There is minor debris build-up along the flanges of the steel.

C. Substructure

The overall condition is good (condition 7). The wingwalls and abutments are in good condition with only very minor cracking and efflorescence noted. There

are no signs of settlement or scour. One weep hole in the south abutment is plugged.

RECOMMENDATIONS

Status of Previous Recommendations

1. Clean debris from deck and bottom flanges Not Done of the girders. Fill, grade and compact rutted areas of the approaches. Remove obstruction from south abutment weep hole.

2. Clean and repaint all structural steel Not done

3. Replace the guardrail support along the Not Done north end of the west guardrail.

Revised Recommendations

Implement above recommendations

Total Updated Estimated Cost \$15,000

STRUCTURES INSPECTION FIELD REPORT

ROUTINE INSPECTION

c.		bridge dept			90-date inspected A July 93
		DKLIN FALS COE 22-owner	<u> </u>	<u> 106-year rebuilt</u>	11-milepoint
2-dist. 104-highway sy	ystem	COE	1945	100 your room	
43-structure type	1		quality control eng	ineer	
ROLED BEA	U BRIDGE	U) TIMBBE OSCI		- FORBES	
07-facility carried			team leader		
•				المدلادر	
06-features intersected	,		team members		
BLAKE F	3frix		M. DESCHE	ES H. WALSH I	BURKHAJER
item 58	7	item 59 SUPERSTRUCTURE	7	item 60_ SUBSTRUCTURE	7
DECK			AL	1. Abutments	
1Wearing Surface	NA	Bearing Devices Stringers	W	a-Wings	7
2. Deck-Condition	8	2. Stringers	NA.	b-Backwa	<u> </u>
3. Stay in Place Forms	, AA	3. Diaphragms	7	c-Bridge S	[]A
4. Curbs	. [7]	4. Girders or Beams	NA.	d-Breastv e-Footing	<u> </u>
5. Median	, NA	5. Floor Beams	96	f-Piles	NA NA
6. Sidewalks	PA	6. Trusses	NA.	g-Erosior	——— I
	JA	7. Rivets or Bolts	اهد ا	h-Settlem	nent 46
7. Parapet	6	8. Welds	1/A	2. Piers or Bents	,
8. Railing	A A	9. Collision Damage	AU	a-Caps	NA NA
9. Anti Missile Fence		10. Load Deflection	ā.∃	b-Column	7 AA
10. Drains	NA	11. Member Alignmen	ו ו	c-Web d-Footing	
11. Lighting Standards	<u> ۲۸</u>	12. Load Vibration	7	e-Piles	
12. Utilities	NA	13. Paint-Epoxy	85 84		PA
13. Deck Joints	PA	14. Year Painted	· · · · · · · · · · · · · · · · · · ·	g-Settlen	
14. Approach Settleme	nt Ma	15. Under Clearance		3. Collision Dama	
, Approach come		Clearance Signs	yes 🚜 no	4. Hydraulic-Adeq	luacy 8
		Cile	Overhead Signs	(attached to bridge)	
Actual Posting	H 3 3S2	Single	yes	X no	
	NW PY WY	10		- 14	
Recommended Posting		15	1. Welds	44	
From Rating Book			2. Bolts	NA	·
SIGNS IN PLACE	at bridge	advance		AG	
YorN	Y		3. Condition	1-1.	
L FOID!! IT\	5			£ -	
LEGIBILITY			Item93b U/W	Inspection Date: บงษ์ย	
ITEM 61-channel and channel protection 7 36-Traffic Safety features					
·			d Briden coll	36	condition
nannel scour 2. embankment erosi		rap or slope paving bractiveness	1. bridge rail 2. transitions		
3. fender system	lon		3. approach	guardrail <u> </u>	7
4. spur dikes & jetties	. ==	etation	4. guardrail t	erminal <u>I</u>	
		-ALA-NOT-APPLIC	ADIE	- INACC	PECCIPI E

PROJECT: FRANKLIN FALLS DAY NAME: BLAKE BROKE BRIDGE LOCATION: HILL, NEW HAMBHIRE

BRIDGE INSPECTION SCOUR CHECKLIST

 Is the bridge currently experiencing, or does it have a history of, scour activity? 	100
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	yes
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	Yes No No No Yes
3. Are any characteristics of an aggressive stream or waterway present?	180
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	20 20 20 20 20 20
4. Is the bridge located on a stream reach with any adverse flow characteristics?	40
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	120 120 120
5. Other comments or observations.	NONE

FRANKLIN FALLS DAM OLD RT 3A BRIDGE, HILL/ BRISTOL, NH FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 14 July 93

<u>DATE OF PREVIOUS INSPECTIONS</u>: Inventory, June 84

Routine, 16 July 87 Routine, 31 May 89 Routine, 14 May 91

RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	11.3T	16.1T	It is recommended that the bridge be limited to 5 tons

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway and Railings: The overall condition is fair (rating

5). The bituminous surface coating is in poor condition with various cracking along the deck. There is vegetation growth and a buildup of debris along both gutters. There are no guardrails at either approach. A safety gate at the north approach is no longer useable. The north approach is rutted with two large potholes approximately 15 feet before the bridge. The south approach is in good condition.

B. Fascias & Curbs:

The parapets on the bridge are in fair condition. There is extensive spalling along the parapet walls. The faces of the walls are covered with graffiti. The anchor bolts supporting the access gate have pulled out of the parapet, and the gate is no longer usable.

C. Underside of Deck:

The overall condition is good. The northern end of the deck diaphragm measured 13" from the breast wall to the back face of the diaphragm (11" 1991 Routine inspection). This would prove that the abutments have moved since the previous inspection. There is some hairline cracking along the concrete tee beams in both transverse and longitudinal directions. The longitudinal cracking

apparent at the approximate center of the east and center tee beams may be due to insufficient cover. Some transverse cracking noted in previous inspections may be caused by excessive shear stresses. The west beam is in good condition.

D. Wingwalls/Abutments:

The overall condition of the wingwalls and abutments is poor (condition 3). The crack in the north abutment appears to have worsened. The crack now measures 5" at top and 1 1/2" at the bottom. The footing is covered in this area but is suspected to be cracked as well. The west wingwall has dropped 1" lower than the breastwall. north abutment is rotated approximately 3 to 5 degrees south and is suspected to have moved since the last inspection. The south abutment has a similar crack at the east side of the breastwall. This crack measures 2 1/4" at the top and 3/4" at the bottom and continues through the footing. The east wingwall has rotated almost 1 3/4" east from the top of the abutment. This abutment has rotated 3 to 5 degrees north. It is difficult to asses whether this wall has undergone any additional movement. abutments appear to have rotated almost 3 inches since the 1984 in-depth inspection and almost 1 foot since construction.

E. Channel:

The overall channel rating is 5. The previous inspection stated that the hydraulic adequacy of the bridge opening is poor. A scour analysis has been performed and is included in the 1992 bridge inspection report appendix A. The area of scour along the south abutment did not appear to be as deep as stated in the 1991 routine inspection.

CONDITION RATING:	1984	1987	1989	1990	1991	1993
	A/E	NED	NED	NH DOT	NED	NED
Deck	6	6	6	7	6	6
Superstructure	7	6	5	7	5	5
Substructure	6	5	5	4	4	4
Channel	N/R	7	. 7	5	5	5
Approaches	6	6	5	4	6	5

Status of Previous Recommendations

1. Post Bridge at 5T gross load to restrict traffic to a pickup truck or less.

Not Done

2. Place guardrail and terminal at the north-west approach

Not Done

3. Place a timber crib to arrest the erosion pass flow adjacent to the northwest wingwall.

Not Done

4. Instrument the cracks, abutments and deck with devices to measure movement more accurately Not Done

Revised Recommendation

Due to the severity of the failure and the apparent movement of the bridge in recent years, total replacement is considered the only practical solution to the problem of the abutment failure. Replacement of the bridge will also allow for an increased load carrying capacity for the bridge. It presently functions as emergency access to the reservoir.

Estimated Cost

\$175,000

Interim Recommendations

1. The bridge should be posted for a 5 ton weight rating and a 10 mph speed limit in order to limit traffic to a pickup truck or light duty dump truck.

Estimated Cost

\$500

- 2. Heavy trucks such as fire apparatus emergency vehicles and light excavation equipment, (backhoe or lighter) should be limited, unloaded, driven slowly, and carefully supervised while travelling over the bridge.
- 3. The street gate presently lying by the bridge should be repaired so that it can be locked. Provisions should also be made so that it will allow pedestrians to cross the bridge easily.

Estimated Cost

\$1000

4. Set reference points and markers in order to monitor the movements of the bridge. Inspect and record movements of the bridge twice per year and include the results of these inspections in the annual bridge inspection report.

Estimated Costs

\$2000 initial survey \$2000 per year

STRUCTURES INSPECTION FIELD REPORT

ROUTINE INSPECTION

		bridge dep	t. no.	8-structu	re no.	90-date inspected
HILL -BRISTOL FRANKL	in FALLS	COF			NEDNH 3310008	
2-dist. 104-highway system	22-owner		27-year t		106-year rebuilt	11-milepoint
X NON-FED	COE		192		NIA	2.2mi Soi 8013781
43-structure type				ontrol eng		
	T- BLAN		team lea		FORBES	
07-facility carried	المراوع مستداد الم	ļ	teamlea		0 mail 22 1	
REC ARM ACCESS ()C 06-features intersected	C RI DM		team m		Small international hypothesist !	
			i	SC HEN	GS /M WALSHILL	BULY MANYER
South Rider			I ION DE	De Fili Lai	7 15: 42- 32- 12:	
item 58 DECK	item 59 SUPERST	RUCTURE		5	item 60 SUBSTRUCTURE	. 4
	1. Bearin	g Devices		4	1. Abutments	4
1. Wearing Surface	2. String	_		NA	a-Wings	
2. Deck-Condition	3. Diaph			7	b-Backwa c-Bridge S	
3. Stay in Place Forms	•	s or Beams		6	d-Breastw	
4. Curbs	5. Floor			NA	e-Footing	
5. Median	6. Truss			NA	f-Piles	₽
6. Sidewalks	7. Rivets			NA	g-Erosion	
7. Parapet	8. Welds			NA	h-Settlem	ent 3
NIA.	-			WA	2. Piers or Bents	
o. naming	Į.	ion Damage		di	a-Caps	<u>A</u> <u>A</u>
G. 7 and Misselle 1 elife		Deflection		. 5	b-Column	
10. Drains		er Alignmen	τ	NA	c-Web d-Footing	
11. Lighting Standards	12. Load			114	e-Piles	NA A
12. Utilities	13. Paint-	•		A 14-	f-Scour	ALA
13. Deck Joints	14. Year		. 10	191	g-Settlem	nent 🏴
14. Approach Settlement 5	15. Unde	r Clearance		in	3. Collision Damag	ge NA
14. Approach Settlement	Clearan	ce Signs	ye	s A no	4. Hydraulic-Adeq	uacy 4
			Overhe	ad Signs	(attached to bridge)	
Actual Posting H 3 3S2	Single		Overne	yes	(attached to bridge)	
	10			•		
Recommended Posting			1. Weld	te	AN	
From Rating Book	5		1. 4461			
			2. Bolts	5	70	
SIGNS IN PLACE at bridge	adva	_	3. Con	dition	AA	
Y or N	. \[\lambda		5. 5611		(
LEGIBILITY	J				nspection Date: Non	¥-
			Item93	D U/VV I	nspection Date: 10015	
ITEM 61-channel and channel protection 5 36-Traffic Safety features						
The containing and charmer protect	ion 5				36	condition
	rap or slope p	paving 5		idge raili		MA 7
	ectiveness	3		ansitions		AC
3. fender system A. spur dikes & jetties A. spur dike	bris getation	D Q		oproach (uardrail t	guardrail <u>น</u> erminal ม	NA.
T. Spui dines a jettles ILM 0. Ve	901411011		7.9		42	

PROJECT: FRANKLIN FALLS

NAME: SLITH RIJER

LOCATION: CLD ET 3A

BRIDGE INSPECTION SCOUR CHECKLIST

 Is the bridge currently experiencing, or does it have a history of, scour activity? 	405
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	yes
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	Yes Yes Yes Nove
3. Are any characteristics of an aggressive stream or waterway present?	YES_
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	465 465 465 UNICIANI
4. Is the bridge located on a stream reach with any adverse flow characteristics?	
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	NO NO NO
5. Other comments or observations.	Nine

BIRCH HILL DAM MIDDLE ROAD BRIDGE, WINCHENDON, MA FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 25 June 93

DATE OF PREVIOUS INSPECTIONS: Inventory, December 84
Routine, September 87

Routine, 18 July 89 Routine, 11 July 91

RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	22T	35T	Load Capacity posted
3	32T	49T	15T (to limit heavy
3 S 2	48T	74 T	truck traffic in
3-3	61T	94T	recreation area)

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway and Railings: Overall rating is 7. Access is limited

by locked gates which prohibit

unauthorized access to the bridge. The buildup of pine needles continues to be a problem since there is restricted traffic over the bridge. The joint sealant at both ends of the prestressed concrete plank has deteriorated. The

concrete plank has deteriorated. The joint sealant has unbonded and the joint is filled with debris. The bituminous surface of the deck is uneven which may cause some minor ponding. The railings

are in good condition. Approach

guardrails are not present and are not warranted due to the restricted access

to the bridge. One bolt on the

guardrail is missing as noted in the

previous inspection.

B. Fascias & Curbs:

Overall rating is 7. The hairline cracks reported in previous inspections and the inventory inspection have not appeared to have worsened. Some efflorescence from the cracks was noted

in this inspection.

C. Underside of Deck:

Overall rating is 7. Minor seepage and efflorescence was noticed between precast concrete planks near the bearing pads. This seepage could be due to water passing through the failed joint sealer and following the joints in the planks. Alignment of the planks is good with no evidence of differential

movement or deflection.

D. Wingwalls/Abutments: Overall rating is 8. The new cast concrete abutments are in good condition with no signs of distress or settlement. No erosion was noted.

E. Channel:

Overall rating is 7. Debris was getting caught under the bridge causing a slight restriction in flow under the bridge.

CONDITION RATINGS

Inventory 1984: Routine 1987: 8 Routine 1989: 7 Routine 1991: Routine 1993:

RECOMMENDATIONS:

Status of Previous Recommendations

1. Schedule annual maintenance to include spot painting of posts, replacement of missing hardware, sweeping deck, and cleaning debris from bridge seat.

Some maintenance done. Must be kept up annually.

2. Reapply sealant at expansion joints Not Done

Revised Recommendations

- 1. Clean expansion joints and reapply sealer to both joints. butyl based or polyurethane based sealant (Sikaflex-15LM or equivalent). Estimated cost \$500.
- 2. Include in annual maintenance, cleaning the debris beneath the bridge from the brook.

- ()				bridge dep	t no	8-structu	re no.	90-date inspected
PalDe 11	U. J. DOAL	WINCHENDO	NUL	COE	. 110.		DWA 251 00 13	6124191
2-dist.	104-highway s		22-owner		27-year	built	106-year rebuilt	11-milepoint
111	NON-FI		CDE		X		1979	
43-structure	e type		_		1 .	ontrol eng		
PREST	ressed coa	LETE SLAG	(COI)			c Foe	Bes	
07-facility c					team lea			
MIDDLE	intersected	Deed beleve	PUBLIC RE	STRETED/	team m		COLUCCI	
_					100000		JES / F. FULL	
TK165	T BROOK				M, L		362 / 1. 1056	
item 58		7	item 59			8	item 60	8
DECK	_		SUPERST	RUCTURE			SUBSTRUCTUF	₹E
	oring Surface	7	1. Bearin	g Devices		8	1. Abutments	s 8
	earing Surface	5	2. Stringe	ers		M	a-Wings b-Backy	
	ck-Condition		3. Diaphi	agms		りな		e Seats
3. Sta	ay in Place Forms		4. Girder	s or Beams		B	d-Breas	. Fal
4. Cu	rbs	7	5. Floor I	Beams		44	e-Footi	-
5. Me	edian	NA.	6. Trusse	es.		NV	f-Piles	1
6. Sic	dewalks	NA	7. Rivets	or Bolts		44	g-Erosi	
7. Pa	rapet	NA	8. Welds	i		AL	h-Settle	
8. Ra	iilina	8	9. Collisi	on Damage		46	2. Piers or Bent	TAN .
	iti Missile Fence	14	10. Load	-		\times	a-Caps b-Colu	
		44		er Alignmer	ıt	8	c-Web	
10. Dr		.18	12. Load	-		44	d-Footi	
11. Lig	ghting Standards	NA	13. Paint-			NA	e-Piles	
12. Uti	ilities		14. Year	· -		NA	f-Scou	
13. De	eck Joints	6		r Clearance	tt ALI	. <u></u> in	g-Settle	
14. Ap	proach Settleme	ent 7		ce Signs		es no	3. Collision Dam	
	•		Olearan	be Oigns	٠, ا		4. Hydraulic-Ade	equacy
					- 			
A		11 0 000	Single		Overhe		(attached to bridge)	
Actual Po	osting	H 3 3S2	511gle	I		yes	no	
			121					
	ended Posting ting Book	22 32 48			1. Wel	ds	44	
Tiomina	ing book				2. Bolt	s	44	
SIGNS I	N PLACE	at bridge	adva	nce			NA	•
Y or N		Y	N		3. Con	dition		
. = 0.15.11	1777	8	<u> </u>	7 -				
LEGIBIL	.1 I Y	<u>U</u>		_	Item93	Bb U/W I	Inspection Date: No	*
/ ITEM	61-channel and	d channel protection	on 8		36-T	raffic Sa	fety features จ	6 condition
Cha	annel scour	8 5. rip :	rap or slope p	avina 🔼	1. bi	ridge raili	i i	I 8
1	bankment eros	ion ይ 6. effe	ctiveness	aving 44		ansitions	_	<u>.</u>
	der system	7. deb		6			guardrail L	
4. spt	ur dikes & jettie	s 🕦 8. veg	etation	P	4. g	uardrail t	erminai <u>'</u>	

PROJECT: BIRCH HILL DAM

NAME: MIDDLE ROAD BRIDGE

LOCATION: WINCHEADD, MA.

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	100
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	yes_
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	Yes Yes No Yes Yes
3. Are any characteristics of an aggressive stream or waterway present?	<u> 130</u>
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	
4. Is the bridge located on a stream reach with any adverse flow characteristics?	No
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	
5. Other comments or observations.	NOYE

BIRCH HILL DAM NEW BOSTON ROAD BRIDGE, WINCHENDON, MA FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 24 June 93

DATE OF PREVIOUS INSPECTIONS: Inventory, 24 September 84

Routine, September 87
Routine, 18 July 89
Routine, 11 July 91

RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	19T	33T	Load Capacity posted
3	24T	40T	15T (to limit heavy
3S2	37 T	62T	truck traffic in
3-3	46T	77 T	recreation area)

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway and Railings: Overall rating is 8. A contract to repair the deck, approaches and railings

was completed in 1992. The deck was

overlaid with a new 2" bituminous paving course. The approaches were also

repaired. New guardrails at each approach were installed. New

elastomeric joint sealer was installed.

Some minor settling and erosion was noticed around some of the new guardrail posts and gabions. There are slight depressions in the approach pavements at the expansion joints which could collect water or create a rough transition onto

the bridge deck.

B. Curbs, Fascias: Overall condition is 7. The concrete in

the curbs and fascias is good. The

minor hairline cracking in the curbs has not appeared to have worsened since the

. previous inspection.

C. Underside of Deck: Overall condition is 8. The concrete in

the precast planks is good. Some minor seepage and efflorescence was noticed on

the underside of the deck along the longitudinal joints and around the bearings. The efflorescence may have

been from previous seepage through the expansion joints prior to replacement of the joint sealer. Alignment is good. The elastomeric bearing pads are also in good condition.

D. Wingwalls/Abutments:

Overall condition is 8. The concrete

cap over the original stone

foundation is in good condition. The

wingwalls have been protected by

installing new gabions which have also helped prevent erosion from runoff from the deck. The erosion on the southwest

bank has been repaired with stone protection and is functioning well.

E. Channel

The overall condition is 8. The

streambed was clear of debris and shows

no sign of scour.

CONDITION RATING

Inventory, 1984 8
Routine, 1987 8
Routine, 1989 7
Routine, 1991 7
Routine, 1993 8

RECOMMENDATIONS:

Status of Previous Recommendations

1. Install "Narrow Bridge" signs.

Not done

2. Install 30'± gabions.

Completed 1992

- 3. Install 75'± guardrail along south- Completed 1992 west approach. Install 45'± guard-rail other approaches.
- 4. Extend transition slabs. Replace Completed 1992 joint filler.
- 5. Schedule maintenance including cleaning sand off bridge, debris off bridge seat, and cut back encroaching vegetation.

Ongoing maintenance

Revised Recommendations

Patch settling and eroding areas around new railing posts. Estimated cost \$500.

City, 10 mm	bridge dep	t. no. 8-structur	re no.	90-date inspected
WHICHENDON, MA BIRCH H			EDMA2510014	6/24/93
2-dist. 104-highway system	22-owner	27-year built	106-year rebuilt	11-milepoint
111 NON-FED	COE	quality control engi		ر_ , _
43-structure type PRESTRESED COX. S	1AB (5AI)	NICH FOR		
07-facility carried		team leader		
	PEC NUESS)	MEPH	COLUCEI	
06-features intersected	J.	team members		
MILLER PIVER		M. DESCH	enes / F. Fun	36
item 58 DECK 1. Wearing Surface 2. Deck-Condition	superstructure 1. Bearing Devices 2. Stringers 3. Diaphragms	8 44	item 60 SUBSTRUCTURE 1. Abutments a-Wings b-Backwa	(B)
3. Stay in Place Forms	4. Girders or Beams	8	c-Bridge d-Breasty	
4. Curbs	5. Floor Beams	44	e-Footing	- CO
5. Median	6. Trusses	44	f-Piles	AA
6. Sidewalks	7. Rivets or Bolts	NA	g-Erosior	
7. Parapet	8. Welds	MA	h-Settlem	nent B
8. Railing	9. Collision Damage	44	2. Piers or Bents	NA
9. Anti Missile Fence	10. Load Deflection	×	a-Caps b-Columi	
10. Drains	11. Member Alignmer	nt S	c-Web	44
70. Dianis	12. Load Vibration	X	d-Footing	
· Lighting Started 5	13. Paint-Epoxy	44	e-Piles	NA.
72. 00	14. Year Painted	NA	f-Scour	PA Nent NA
2	15. Under Clearance	<u> </u>	g-Settlen	
14. Approach Settlement	Clearance Signs	yes NN no	Collision Dama Hydraulic-Adeq	
			(
Actual Posting H 3 3S2	Single	Overhead Signs ((attached to bridge)	
Recommended Posting		1. Welds	44	}
From Rating Book		,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,,	NA	
SIGNO IN DI AGE	c.4a-c-	2. Bolts		
SIGNS IN PLACE at bridge Y or N	advance	3. Condition	NA	
LEGIBILITY		Item93b U/W Ir	nspection Date:	E
1TEM 61-channel and channel protect	ion 8	36-Traffic Saf	ety features	con <u>dit</u> ion
2. embankment erosion	rap or slope paving ectiveness bris getation	1. bridge railir 2. transitions 3. approach g 4. guardrail te	ng 1 guardrail 1	8 3 3

PROJECT: BIRCH HILL DAM

NAME: NEW BOOTEN PORD

LOCATION: WINCHESDEN, MA

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	yes
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	Ye5
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	Yes Yes No No Yes
3. Are any characteristics of an aggressive stream or waterway present?	<u>Nes</u>
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	NO Yes Yes NO
4. Is the bridge located on a stream reach with any adverse flow characteristics?	yes_
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	No Ves Ves
5 Other comments or observations.	NONE

BIRCH HILL DAM BURGESS ROAD BRIDGE, WINCHENDON, MA FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 24 June 93

DATE OF PREVIOUS INSPECTIONS: Inventory, 24 September 84
Routine, September 87
Routine, 18 July 89

Routine, 21 September 90

RATING (T=TONS)

Туре	Inventory	Operating	Comments
H15	30T	47T	Load Capacity posted
3	43T	66 T	15T (to limit heavy
3 S 2	66T	101T	truck traffic in
3-3	84T	128T	recreation area)

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway and Railings: Overall rating is 7. There are several

depressions and ruts in the deck and approach slab pavements. There is moderate vegetation growth and pine needles along both curbs. Vegetation at the southwest corner of the bridge has not been removed and is encroaching into the roadway as mentioned in previous inspections. The pavement at the expansion joints along the west approach sinks below the concrete edges Guardrails should be installed at each corner of the approaches as noted in previous inspections in order to ensure adequate safety for approaching vehicles.

B. Fascias and Curbs:

Overall condition is 7. The concrete in the curbs and fascias is in good condition. The hairline cracks in the curbs show some efflorescence and do not seem to have deteriorated since the last inspection. The approach curb at the south west corner of the bridge is cracked along the top which may eventually propagate into a spall.

C. Underside of Deck:

Overall condition is 7. The concrete in the precast planks is in good condition. The spall mentioned in the 1991 report could not be located, however the general area should be continually inspected in order to determine if there may be any deficiency in the concrete planks. There has been no change in the condition of the one inch differential between the precast concrete planks. Some seepage and efflorescence was noticed between the concrete planks near the bearing pads.

D. Wingwalls/Abutments:

Overall condition is 7. The concrete caps over the original rubble masonry are good. The elastomeric bearing pads are also in good condition. The

abutments show no signs of settlement,

deterioration or scour.

E. Channel:

The overall condition of the channel is 7. The brook was flowing smoothly, however, debris was building up under the bridge, creating a slight

obstruction to flow.

CONDITION RATING

Inventory, 1984 8
Routine, 1987 8
Routine, 1989 7
Routine, 1991 7
Routine, 1993 7

RECOMMENDATIONS

Status of Previous Recommendations

1.	Install	"Narrow	Bridge"	warning	Not	done
	signs.		_	_		

2. Install guardrail at approaches Not done

3. Extend transition slabs, install Not done drainage, and seal expansion joints.

Revised Recommendations

Install 75' \pm of guardrail at the approach at the southwest wingwall and remove encroaching vegetation. Install 45' \pm of guardrail at each of the other three corners of the bridge. 3"x10" pressure treated rails with 8"x8" pressure treated posts are recommended. Replace the joint sealant in the expansion joints. Estimated cost \$7500.

c C				bridge dep	t. no.	8-structu	re no.	90-	date inspected
WINKH	ENOW, MA	BIRCH HIL	L DAM			CEPNE	DMA 2510015		0124193
2-dist.	104-highway sy		22-owner		27-year t		106-year rebuilt	11-1	milepoint
111			COE		197		inoot		
43-structure		:0c- 6100	1000			ontrol engi	FOLBES		
	PRESTRESSED CONCRETE SLAG (501) 07-facility carried				team lea				
BIRG						COLU	16.6.1		
06-features					team m				
PRIE	-ST BROOK	<u></u>			M. DE	SC HEN	ES / F. FUNG		
item 58	_	7	item 59			8	item 60		8
DECK		<u></u>	SUPERST	RUCTURE			SUBSTRUCTUR	RE	
1. We	aring Surface	. 7	1. Bearing	g Devices		8	1. Abutments	_	
	ck-Condition	7	2. Stringe	rs		NA	a-Wing b-Back		8
		MA	3. Diaphr	agms		44		e Seats	
	y in Place Forms	7	4. Girders	or Beams		8	d-Breas		8
4. Cur	rbs		5. Floor E	Beams		AL	e-Footi	ngs	8
5. Med	dian	NA.	6. Trusse	s		NA NA	f-Piles		ALA
6. Sid	ewalks	ALA	7. Rivets	or Bolts		NA	g-Erosi	ion	8
7. Par	rapet	NA	8. Welds			44	h-Settle		8
8. Rai	ilina	7		n Damage		NK	2. Piers or Bent		
	ti Missile Fence	NA	10. Load D	•		-	a-Caps		17 <u>7</u>
		ALI		er Alignmen		7	b-Colu c-Web	mn	44
10. Dra		AA.	12. Load V	•	•		d-Footi	ina	NA AL
11. Ligi	hting Standards					71	e-Piles	-	<u>AA</u>
12. Util	lities	AL	13. Paint-E			44	f-Scou		NA
13. Dec	ck Joints	6	14. Year P		. 11.		g-Settle	ement	NA
14. Apr	proach Settlemen	6		Clearance _			3. Collision Dam	nage	लेक
	•		Clearanc	e Signs	yes	NA no	4. Hydraulic-Ade	equacy	3
				<u> </u>	Overboo	d Signs /	attached to bridge)		
Actual Pos	sting	H 3 3S2	Single		Overne	yes	no		
			150		<u>-</u>	•			
Recomme	ended Posting				1. Weld	•	44		
From Ratio	ng Book	30 43 66			1. ***	· ·	- NA		i
					2. Bolts		100		
SIGNS IN	N PLACE a	at bridge	advan	ce	3. Cond	ition	NA		
Y or N		4	μ.		o. cond	11.011	<u> </u>		
LEGIBILI	TY	8		_			espection Date: Non	JE_	
					Item93b	U/W In	spection Date: Dor		
TEM 6	S1-channel and (channel protectio	n 👩		36-Tr	affic Safe	ety features		
1	, ondiner and t	Sharmer proteotio	<u> </u>		30-11	unio Dale	ety leatures <u>36</u>	6 (condition
	nnel scour	8 5. rip ra	ap or slope pa	aving DA		dge railin			7
	ankment erosio		ctiveness	25		nsitions	<u> C</u>	_	<u></u>
	er system dikes & jetties	NA 7. debr		NA.	, , ,	oroach gi			
4. spui	aines a jeules	rej o. vege	ταιιστι	1412	4. gu	ardrail tei	milital		

PROJECT: BIRCH HILL
NAME: BURGESS PD
LOCATION: WINCHES DAY, MA

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	NO
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	Yes_
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	yes yes yes
3. Are any characteristics of an aggressive stream or waterway present?	٥٥
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	NO NO NO NO NO
4. Is the bridge located on a stream reach with any adverse flow characteristics?	ظم
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	りり いっ いっ いっ
5. Other comments or observations.	NOVE

BIRCH HILL DAM OLD ROUTE 202 BRIDGE, WINCHENDON MA. FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 24 June 93

DATE OF PREVIOUS INSPECTIONS: Inventory, 24 May 84.

Routine, Sep 87.

Routine, 29 Jul 89.

Routine, 21 Sep 90.

RATING (T = TONS)

Type	Inventory	Operating	Comments
H15	23T	35T	Load rating
3	34T	53T	recalculated due
3S2	54T	84T	to new deck
3-3	66 T	103T	concrete overlay.

A. Roadway and Railings

Overall rating 7. The bridge west approach showed some depression but the overall transition to the concrete deck is smooth. The approach guardrail, bridge rails, concrete overlay and transition slabs are in good condition. The approach guardrail are far from the pavement but they are functional. Slight erosion is located at the southwest and northeast approach corner.

B. Fascias

Overall condition is 8. Both fascia and bridge deck are in good condition. No cracks or concrete spall were located. Bridge deck is also in excellent condition. The deck drainage and weep holes are clear.

C. Underside of Deck

Overall condition is 8. The beams and deck diaphragm do not have any sign of concrete spall. No cracks or water staining was noted. The underside of the deck is in good condition.

D. Wingwalls & Abutments

Overall condition 7. The wingwalls are in good condition. There are is erosion at the bottom of bituminous waterway behind southeast wingwall. Also minor spall on the southwest corner of abutment with moss growth was noted. There are no signs of scour at the foundation.

E. Channel

Overall condition 7. The streambed under the bridge is filled with vegetation and tree branches. The stream flow was moderate during inspection; however, no major signs of scour were noted.

CONT	ጎፐጥፐ /	ON RA	TING
COM) [] [[(JN KA	TING

Inventory	Y	6
Routine,	1987:	6
Routine,	1989:	5
Routine,	1991	7
Routine,	1993	7

RECOMMENDATIONS:

Status of Previous Recommendations

 Modify approach guardrail at transitions to guide around brush blocks.

Not done

2. Make miscellaneous patch repairs to abutments and wingwalls at flaws which were missed in 1990 contract or below the existing water level (contractor limit of work).

Not done

Revised Recommendations

 Remove all the tree branches, debris and other vegetation near and under the bridge deck. (Project Personnel)

	WINCHENDO	N, MA		bridge dep	t. no.	8-structure	e no.	90-date inspected 6 / 24 / 93
2-dist.	104-highway system		22-owner COE	<u>!</u>	27-year t		106-year rebuilt 1990	11-milepoint
43-structur		/ MULTI .	-BEAM OR	GIRDER	quality co	ontrol engin	neer	
07-facility		ROUTE	202		team lea		Colucci	
06-feature	s intersected WATER	BEAUER	R BROO	K	team m		SCHENES /F	. FUNG
2. De 3. St 4. Ci 5. M 6. Si 7. Pi 8. R 9. A 10. D 11. Li 12. U 13. D	rearing Surface eck-Condition tay in Place Forms urbs edian idewalks arapet ailing nti Missile Fence trains ighting Standards	8 8 - 8 - 8 8 8	1. Bearin 2. Stringe 3. Diaphr 4. Girder 5. Floor I 6. Trusse 7. Rivets 8. Welds 9. Collisi 10. Load I 11. Memb 12. Load I 13. Paint- 14. Year I 15. Under	ragms s or Beams Beams es or Bolts on Damage Deflection per Alignmer Vibration Epoxy		8 - 8 8 8 - 1 - 2 - 3 - 3 - 3 - 3 - 3 - 3 - 3 - 3 - 3 - 3	item 60 SUBSTRUCTURE 1. Abutments a-Wings b-Backwa c-Bridge S d-Breastv e-Footing f-Piles g-Erosior h-Settlem 2. Piers or Bents a-Caps b-Column c-Web d-Footing e-Piles f-Scour g-Settlen 3. Collision Dama 4. Hydraulic-Adeq	All Seats Seats vall 8 seats s
	mended Posting	3 3S2 6 69 7Z 2 — —	Single		Overhe	yes	attached to bridge) no	
	ating book	ridge	adva	nce	2. Bolts 3. Con			
LEGIBI	LITY C	<u>7</u>]	Item93	b U/W In	spection Date:	
1. ch 2. en 3. fei	1 61-channel and cha nannel scour nbankment erosion nder system our dikes & jetties	\$ 5. rip6. effe7. deb	rap or slope pectiveness	paving – 6	1. br 2. tra 3. ap	raffic Safe ridge railin ansitions oproach g uardrail te	uardrail 0	condition 8 7 8

PROJECT: BIRCH HILL DAM

NAME: OID ROUTE 202

LOCATION: WINCHENDON, MA

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	YES
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	YES YES YES NO YES —
3. Are any characteristics of an aggressive stream or waterway present?	YES
a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. Concrete At Waterline	NO NO YES NO YES
4. Is the bridge located on a stream reach with any adverse flow characteristics?	
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	No No No YES
5. Other comments or observations.	NONE

BIRCH HILL DAM GOODNOW ROAD BRIDGE, WINCHENDON, MA FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 25 June 93

25 September 84 <u>DATE OF PREVIOUS INSPECTIONS</u>: Inventory, Routine, 4 September 87 4 September 87 Deck reinforcing inspection, Routine, 19 July 89 21 September 90 Routine,

RATING (T=TONS)

Operating Comments Inventory Type 18T No change in ratings H15 13T

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway and Railings: Overall condition is good, no repairs needed (condition 8). The bituminous concrete deck overlay and transition

slabs are in good condition. All deck drains are clear and functioning

properly. The approach alignments are only 16 feet wide and slightly skewed. The timber approach and bridge railings

are in good condition.

B. Fascias & Curbs: The fascias and curbs are in good

condition.

C. Underside of Deck: The overall condition of the underside of the concrete deck is good. One spall

was noted in the concrete deck at approximately the third point of the outside east beam. The spall is approximately 12" long, 4"wide, and 4" deep. This spall has been noted in previous inspections, has not continued to deteriorate, and is not a concern at

this time.

The condition of the abutments and D. Wingwalls/Abutments: wingwalls is good (condition 7). There

are only minor hairline cracks with efflorescence on the east face of the north abutment. All other concrete is in good condition. The gabion retaining

walls are in good condition. Erosion was again noted beneath the south

abutment, and should be repaired.

E. Channel:

Overall condition 7. There is an

existing area of scour beneath the south

abutment. The flow beneath the bridge

was swift with little obstruction.

Some minor rubble is deposited under the

bridge.

CONDITION RATINGS:

Inventory, 1984: 7
Routine, 1987: 7
Routine, 1989: 6
Routine, 1991: 7
Routine, 1993: 7

Routine, 1995.

Status of Previous Recommendations

Item

Current Status

 Post warning signs "Narrow Bridge" on both approaches. Not Done

2. Repair scour at abutments.

Not Done

Revised Recommendations

Repair scour at abutments.

Estimated cost \$5,000.

P. C.				bridge dep		8-structu	re no. EDNA 251 0017	90-date inspected 6/24/93
2-dist.	104-highway syste		22-owner	1 COE	27-year l		106-year rebuilt	11-milepoint
111	NON-FE		COE			37	1991	0,5
43-structure					quality co	ontrol eng	ineer	
MULTIS	BEAM I CONE	LETE DECK	(302)		NIC	V FOR	865 	
07-facility ca					team lea	ader		
60000	WW RD (R	EC MEA A	uss)				DICKLI	
06-features	intersected		•		team m		1	
PRIE	ST BROOK				I LL, DE	SCHEN	ES / F. FUNG	
item 58		8	item 59			8	item 60 SUBSTRUCTURE	7
DECK			SUPERST	RUCTURE			1. Abutments	
1 Wes	aring Surface	8	1. Bearin	g Devices		8	a-Wings	7
	k-Condition	8	2. Stringe	ers		8	b-Backwa	. 5750
		NT S	3. Diaphr	agms		8	c-Bridge S	
	y in Place Forms	8	4. Girder	s or Beams		46	d-Breastw	rall 8
4. Cur	bs		5. Floor I	Beams		NA	e-Footing	
5. Med	dian	14	6. Trusse	es		AG	f-Piles	70
6. Side	ewalks	12%	7. Rivets	or Bolts		AK	g-Erosion	
7. Par	apet	MA	8. Welds			44	h-Settlem	ent 8
8. Rai	lina	8	9. Collisi	on Damage		27	2. Piers or Bents	20
,	i Missile Fence	44	10. Load I	•		\times	a-Caps b-Column	
		8		er Alignmen	t	8	c-Web	ALA
10. Dra		124	12. Load	-		X	d-Footing	
11. Ligi	hting Standards		13. Paint-			8	e-Piles	44
12. Util	ities	L'A.	14. Year I	· · ·	1990		f-Scour	44
13. Dec	ck Joints	8		Clearance		in	g-Settlem	
14. Apr	oroach Settlement	8	l.				3. Collision Damag	· — i
			Clearand	se Signs	~) ye	5 110	4. Hydraulic-Adeq	uacy 🗂
Actual Pos	stina	H 3 3S2	Single		Overhe	ad Signs yes	(attached to bridge)	
	-	ALD ALD ALD	12:1	1		yes	110	
Pocommo	ended Posting						40	
From Rati			137		1. Weld	ds		
					2. Bolts	5	NA.	
SIGNS IN	NPLACE at	bridge	advar	ice	0.0	-1141	NA	
Y or N			h]	3. Con	dition	<u>(</u>	
LEGIBILI	TV			1 -			nspection Date: No.26	
ELGIDILI					Item93	b U/W I	nspection Date: None	•
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NAME: 600 NOW PD LOCATION: WIND CHEBON, HA

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	yes
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	yes_
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	yes yes no no yes
3. Are any characteristics of an aggressive stream or waterway present?	<u>yes</u>
a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. Store and all the store and the store.	yes no yes no yes
4. Is the bridge located on a stream reach with any adverse flow characteristics?	<u>yes</u>
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	00 yes yes
5. Other comments or observations.	NONE

WEST HILL DAM WEST HILL ROAD BRIDGE, UXBRIDGE, MA FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 8 September 93

DATE OF PREVIOUS INSPECTIONS: Inventory,

Routine,

23 August 89 30 July 91

RATING (T=TONS)

Type	Inventory	Operating	Comments
Н15	12T	24T	No change in ratings Ratings based on satisfactory past performance without signs of distress.

EVALUATION (See attached "Structures Inspection Field Report")

A. Deck, Roadway & Railings:

Overall condition is 7. The roadway over the bridge is in good condition. Slight vegetation buildup was noticed along the granite curbs. The pavement along the northeast, and southwest wingwalls is beginning to erode due to runoff from the road. Steel quardrail sections that were installed along the northwest approach in order to control erosion are performing satisfactorily. The railings along the bridge deck are in good condition, however they are also very light duty and do not comply with the current AASHTO standards. There is a poor transition between the approach guardrails and the bridge deck railings along the north approach. The cables for the north approach guardrails are There are no guardrails loose. along the south approach. speed bumps at either end of the bridge are effective in controlling the speed of traffic. The bridge which is 18 feet wide is narrow and is currently used for two way traffic and pedestrians.

B. Superstructure/ Substructure

The overall condition is good (condition 8). The stones seem to be well bonded and aligned. is no sign of distress of the superstructure. The mortar grout on the underside of the arch is delaminating and spalling. It appears that during the 1940 rehabilitation of the bridge, the underside of the arch was formed in order to contain the flow of grout which was pressure injected from above the arch into the joints in the stone. In this case the thin mortar layer does not provide any additional structural strength and therefore the delaminating mortar is not a concern. There is some moss and vegetation growing from the joints between the stones. Some of the joints should be cleaned and repointed. superstructure is primarily integral with substructure. substructure is also in good condition. Due to the depth of the water, the footings were not examined for scour potential.

C. Channel:

It was difficult to evaluate the overall condition of the channel. There was very light flow through the bridge at the time of the inspection. No erosion was noticed in the channel. The upstream channel makes a sharp turn south and another turn west before reaching the bridge.

CONDITION RATING:

1989	8
1991	8
1993	7

RECOMMENDATIONS:

Status of Previous Recommendations

1. Perform mortar joint repairs. Remove vegetation and repoint the joints over the stone arch on both sides.

Not Done

2. Control erosion and stabilize the west embankment.

Not Done

3. Install new approach and bridge guardrails.

Not Done

4. Install a pedestrian walkway.

Not Done

Revised Recommendations

A contract has been prepared during FY 93 to perform the above recommendations. No work had yet been accomplished by the time of the inspection.

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PROJECT: WEST HILL DAM
NAME: WEST HILL DAM DELDGE
LOCATION: UKPLINGE, MA

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	yes
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	yes
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	Yes Yes No Yes
3. Are any characteristics of an aggressive stream or waterway present?	- Yes
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	NO NO NO YES UNKIDIO 1936-1940 CEMO?
4. Is the bridge located on a stream reach with any adverse flow characteristics?	<u>Yes</u>
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	NO NO NO NO
5. Other comments or observations.	NONE

THOMASTON DAM LEADMINE BROOK ROAD BRIDGE, THOMASTON, CT FISCAL YEAR 1993 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION:

24 August 93

DATE OF PREVIOUS INSPECTIONS:

Routine Inspection, 16 June 91 Inventory Inspection, November 84

RATING (T = TONS)

Туре	Inventory	Operating	Comments
H15	11T	16T	No change in
Type 3	45T	69T	ratings due to
Type 3S2	69T	106T	inspection findings.
Type 3-3	86T	132T	

EVALUATION (See attached "Structures Inspection Field Report")

A. Superstructure -Above Deck

- -Overall condition is very good.
- -There are a few small potholes in the east approach pavement.
- -All of the approach stone walls are in good condition, with only minor vegetation growth between the pavement and the base of the wall.
- -The concrete transition slab at the east approach is in good condition.
- -The expansion joint at the east approach is in good condition.
- -The pavement at the west approach has a few bumps at the transition onto the deck.
- -Bridge railings and posts are in good condition. There are some minor shrinkage cracks in the surface coats of the concrete posts.
- -There is sand and debris accumulating on the deck near the south curb.
- -There are a few patches of deterioration in the bituminous wearing surface.
- B. Superstructure -Below Deck
- -Overall condition is good.
 - -The structural steel has recently been painted (1990), and is in good condition.
 - -Underside of deck is in good condition, with only minor honeycombing. There is some minor efflorescence coming from several transverse hairline cracks beneath both curbs.

Substructure C.

- -Overall condition is good.
- -The stone abutments are in good condition, with no signs of movement or settlement.
- -All of the four stone wingwalls are in good condition, with no signs of movement.
- -The east abutment is slightly undermined by scour.

D. Channel

- -The channel is scouring beneath the bridge. The channel is approximately four feet deeper under the bridge than it is either upstream or downstream of the bridge.
- Overall Numerical E. Condition Rating

Inventory 1985: Routine 1991: 8 Routine 1993:

RECOMMENDATIONS

Status of Previous Recommendations

- Inspect both abutments for scour. 1.
- Repair scour erosion at the south corner of the east abutment. 2. None of this work has been done.

Revised Recommendations

- Complete the scour analysis of the east abutment. The west 1. abutment is founded on rock and it is unlikely that it is susceptible to scour.
- Post a 10 Ton load limit at the east approach. 2.

Thomaston CT	bridge dept		e no. DC <i>T0910003</i>	90-date inspected
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SCOUR CHECKLIST

1. Is the bridge <u>currently</u> experiencing, or does it have a history of, scour activity?	<u> </u>
2. Is streambed erodible? If so, does the structure have any vulnerable design features?	<u>/es_</u>
 a. Piers, abutments with <u>spread footings</u> or short pile foundations. b. Superstructures with <u>simple spans</u> or non-redundant support systems. c. Inadequate waterway opening. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	
3. Are any characteristics of an aggressive stream or waterway present? a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other.	
4. Is bridge located on stream reach with any adverse flow characteristics?	
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	
E Other comments or observations	

NORTHFIELD BROOK LAKE BRIDGE ON OLD ROUTE 254 (UPPER), THOMASTON, CT FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 24 August 1993

DATE OF PREVIOUS INSPECTIONS: In-depth, Dec 84

Routine, Aug 87 Routine, Aug 89 Routine, June 91

RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	19T	28T	
3	34T	52T	
3S2	49T	52 T	

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway & Railings

The overall condition of the deck and railings is good (condition 7). The upper cable of the north east approach is loose and tangled. The remaining cable guardrails along the roadway are in very good condition. Both bridge railings are in good condition. Most concrete spalls have been patched. One repair in the south end of the east rail has spalled due to wood forming remaining in the patch. The deck and approaches are in good condition. Various areas of the deck appear to have been filled with bituminous patching.

B. Fascias and Curbs

The condition of the fascias and curbs is good. The concrete shows evidence of abrasion typical of aged concrete. Of minor concern is the lack of joint filler between curb monoliths. There is a buildup of debris in some of these joints.

C. Underside of Deck

The underside of the deck is in very good condition (condition 8) and appears to have been recently painted. The bearings and underside of the concrete deck are

in good condition with no signs of distress or deterioration.

D. Wingwalls and Abutments

The wingwalls and abutments are in good condition (condition 7).

Moderate abrasion is typical of all walls. One minor vertical crack was noted in the south east wingwall and minor efflorescence noted on the west walls. The weep holes in the south abutment are clear and appear to be functioning properly. The weep holes in the north abutment were buried under sand deposited against the wall.

E. Channel

The channel is undergoing various amounts of erosion. Although no scour below the bridge footings was noted, moderate aggredation was present along the north abutment. Both upstream and downstream of the bridge, dense vegetation was encroaching upon the channel.

CONDITION RATING

In-depth 7
Interim 1987 7
Interim 1989 7
Routine 1991 8
Routine 1993 7

RECOMMENDATIONS

Status of Previous Recommendations

No Previous recommendations

Revised Recommendations

The deficiencies noted are not of much concern at this time. They may be combined with repairs to other local bridges in the future.

				bridge dep	t. no.	8-structur		90-date inspected
THOM ASTON CT. 2-dist. 104-highway system 22-owner			00	COE.	07 voor h	27-year built 106-year reb		8/23/93 11-milepoint
2-dist.	104-highway system		22-owner		UNKN	. 1	106-year rebuilt	Trannepoint
43-etructure	LOCAL		مرسيا من ا			_		
	G-structure type quality control engineer STEEL SINGLE SPAN WIDE FLANGE BEAM NICK FORRES							
07-facility c					team leader			
	D RT 254	 			JOE COLUCCI			
06-features	intersected				team members			
NOR	THEIELD BROO	<u> </u>			IM DE	5 CH678	S, M WALSH,	M JORID
item 58		7	item 59			B	item 60 SUBSTRUCTURE	7
DECK			SUPERST	RUCTURE			1. Abutments	
1. We	earing Surface	7	1. Bearin	g Devices		8	a-Wings	7
	ck-Condition	7	2. Stringe	ers			b-Backwa	
	· .	-	3. Diaphr	ragms		8	c-Bridge S	
	ay in Place Forms	7	4. Girder	s or Beams		3 60 1	d-Breastv	vall 🗇 🔭
4. Cu			5. Floor I	Beams			e-Footing	s <u>7</u>
5. Me	edian		6. Trusse	es			f-Piles	
6. Sic	iewalks		7. Rivets	or Bolts		8	g-Erosion	
7. Pa	rapet	7	8. Welds	;		_	h-Settlem	ent <u> </u>
8. Ra	illing	7	9. Collisi	on Damage			2. Piers or Bents a-Caps	
9. An	ti Missile Fence	_	10. Load I	Deflection			b-Column	
10. Dra	ains	<u>-</u>]	11. Memb	er Alignmen	it	8	c-Web	
11. Lig	hting Standards	_	12. Load	Vibration			d-Footing	
12. Uti	_	_	13. Paint-	Epoxy		9	e-Piles	
i	eck Joints	-	14. Year I	Painted		9_1	f-Scour	
		8	15. Under	r Clearance	ft .	in	g-Settlen 3. Collision Dama	1
14. Ap	proach Settlement	<u> </u>	Clearand	ce Signs	ye	s MA no	4. Hydraulic-Adeq	
							,	
A atual Da	ostina H	3 3S2	Single		Overhe	ad Signs	(attached to bridge)	
Actual Po	osung A	3 332				yes	X no	
Recomm	ended Posting						AN	
From Rat	9 1 11				1. Weld	ds		
					2. Bolts	3	NA	
1	N PLACE at bride	ge	advar		3. Con	dition	NA	
Y or N	N		N]	J. 0011	dition	<u> </u>	
LEGIBIL	ITY Y		N		Item93	h II/W I	nspection Date: Not	YE.
ITEM 61-channel and channel protection 36-Traffic Safety features 36 condition					condition			
hannel scour S 5. rip rap or slope paving				1. br	1. bridge railing			
1			ectiveness	-	2. tra	ansitions	1	1
i	der system	7. deb		5		oproach (
4. Spu	4. spur dikes & jetties 8. vegetation 4. guardrail terminal							
X=U	NKNOWN	en de l'accession	NA=NOT	APPLIC	ABLE		IA=INACC	ESSIBLE

PROJECT: NORTH FIELD BROOK LAKE NAME: LITTLE RT 254 BRIDLE LOCATION: OLD RT 254 (UPPER)

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	YES
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	YES YES NO NO YES
3. Are any characteristics of an aggressive stream or waterway present?	YES
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	765 NO NO
4. Is the bridge located on a stream reach with any adverse flow characteristics?	<u>YES</u>
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	10 10 10 10 10
5. Other comments or observations.	

NORTHFIELD BROOK LAKE BRIDGE ON OLD ROUTE 254, (LOWER) THOMASTON, CT FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 24 August 1993

DATE OF PREVIOUS INSPECTIONS: In-depth, Dec 84

Routine, Aug 87 Routine, Aug 89 Routine, Sept 91

RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	16T	23T	
3	39T	55 T	
3S2	62T	86T	

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway, & Railings

The overall condition of the roadway, railings and approaches is good. The wearing surface of the deck has been recently replaced. Cracks were noted across the deck at approximately 8 to 10 foot intervals. The cracking appears to be the result of improper curing. The cable roadway guardrails are in good condition. One cable along the south east approach is loose. The bituminous approaches have been repaired recently. The new approaches are slightly higher than the deck causing a slight impact when entering and exiting the bridge. The railings at each approach are in good condition. The west guardrail shows some abrasion of the concrete, typical of its age.

- B. Curbs and Fascias
- The curbs and fascias along both sides of the deck are in good condition with no apparent signs of distress or deterioration.
- C. Underside of Deck

The overall condition of the superstructure below the deck is good. Three of the T-beams on the

east side of the bridge have minor spalls and minor to moderate efflorescence. The two inner T-beams are in very good condition. The two west steel beams are in good condition. There is minor rusting apparent on the underside of the deck from the reinforcement chairs. The bearings for both the steel and concrete beams are in good condition.

D. Wingwalls and Abutments

The wingwalls and abutments are in good condition. Only minor cracking and efflorescence was noted along the wingwalls.

E. Channel

The bridge is located at the end of a bend in the channel. There is some aggredation of the channel along the north abutment. The downstream side of the channel is clear.

CONDITION RATING

In-depth 7
Routine 1987 7
Routine 1989 6
Routine 1991 8
Routine 1993 7

RECOMMENDATIONS

Status of Previous Recommendations

No previous recommendations.

Revised Recommendations

No new recommendations at this time.

	oridge dept. n	*****		90-date inspected	
THOMASTON CT. CUE				VEDCT091 0005	24 AUGUST 93
2-dist. 104-highway system	22-owner こので、	27	7-year built 1936	106-year rebuilt	11-milepoint
43-structure type		quality control engineer			
SIMPLE SOAN CONYPORTE T-BEAM	s / STEEL 514	1.	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		
07-facility carried			team leader		
OLD ROUTE 254 (L	OWER)		JOE COLUCKI		
06-features intersected	-		team members		
NORTHFIELD BROOK.		<i>l</i> .	1. DESCH	ENTER MIWALSH.	MILORIO
item 58	item 59 SUPERSTRI	UCTURE	7	item 60 SUBSTRUCTURE	7
DECK 1. Wearing Surface	1. Bearing I		7	1. Abutments	
1. Wearing ounded	2. Stringers			a-Wings	
2. Deck-Condition	3. Diaphrag		[7]	b-Backwa	
3. Stay in Place Forms	4. Girders of		$\overline{7}$	c-Bridge \$ d-Breastw	
4. Curbs	5. Floor Be		-	e-Footing	
5. Median	6. Trusses	-	_	f-Piles	-
6. Sidewalks	7. Rivets or	r Bolts	-	g-Erosion	
7. Parapet	8. Welds			h-Settlem	ent
8. Railing	9. Collision	Damage	_	2. Piers or Bents a-Caps	-
9. Anti Missile Fence	10. Load De	•	. –	b-Column	
10. Drains	11. Member	Alignment	ڪ	c-Web	~
11. Lighting Standards	12. Load Vit	oration		d-Footing	<u> </u>
12. Utilities	13. Paint-Ep	ооху	<u>8</u>	e-Piles	nent —
	14. Year Pa	inted	<u> 1/</u>	f-Scour	<u></u>
13. Deck Joints	15. Under C	learance	ft	in g-Settlen 3. Collision Dama	
14. Approach Settlement	Clearance	Signs	yes IIA	no 4. Hydraulic-Adeq	
•		•		4. Hydraulio 7 dod	
			Overhead Sig	ns (attached to bridge)	!
Actual Posting H 3 3S2	Single		yes	no	
Recommended Posting From Rating Book	_		1. Welds		
From hatting book			2. Bolts	-	
SIGNS IN PLACE at bridge	advanc	e			
YorN	N		3. Condition		
LECIDILITY	-				
LEGIBILITY			Item93b U/	W Inspection Date:	
ITEM 61-channel and channel protection 36-Traffic Safety features					
				36	condition 7
	rip rap or slope pav effectiveness	ving -	1. bridge ra 2. transitio		ˈ
	debris	7	1	ch guardrail	<u> </u>
4. spur dikes & jetties - 8.	4. guardra				
X-UNKNOWN	NA=NOT	APPLICA	BIF	JA=INACC	ESSIBLE

PROJECT: NORTH FIELD BROOK NAME: OD DT ZSA (LOWER) LOCATION: OLD PT ZSA

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	YES
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES_
a. Piers, abutments with spread footings or short pile foundations.	yes
b. Superstructure with simple spans or non-redundant support systems.	Yes
c. Inadequate waterway openings. d. Designs which collect ice and debris.	<u> </u>
e. All water must pass through or over structure. f. Other.	<u> YE5 </u>
3. Are any characteristics of an aggressive stream or waterway present?	yes
a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of	405
streambanks.	20
c. Steep slopes. d. High velocities.	<u>50</u>
e. Any history of highway or bridge damage during past floods.f. Other.	70
4. Is the bridge located on a stream reach with any adverse flow characteristics?	VES
a. Crossing near stream confluence.	NO
b. Crossing of tributary stream near confluence with larger streams.	70
c. Crossing on sharp bend in stream.d. Location on alluvial fan.	<u> </u>
e. Other.	
5. Other comments or observations.	Strele

BLACK ROCK LAKE OLD NORTHFIELD ROAD BRIDGE, THOMASTON, CT FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 24 August 1993

<u>DATE OF PREVIOUS INSPECTIONS</u>: In-depth, Dec 84

Routine, Aug 87 Routine, Aug 89 Routine, June 91

RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	11T	16T	Ratings similar to
3	25T	40T	those determined in
3 S 2	39 T	63T	the 1984 in-depth
3-3	49T	78T	report.

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway, Railings, and Deck.

The general condition is good (condition 8). The bituminous wearing surface on the north approach and south approach is in good condition. The transitions to the deck from the north and south approaches are not smooth. The expansion joint is sealed and in adequately good condition. The concrete bridge deck is in good condition. The scuppers are clear. The rails on the bridge deck are in good condition. There is some minor vegetation growth at the curbs on the bridge deck. The approach guardrails are in good condition.

B. Fascia and Curbs

The general condition is good (condition 8). The overall condition of the curbs is good; they have recently been painted. There is a minor crack at the northeast corner of the curb and some minor honeycombing.

C. Underside of Deck and Bearings.

The overall condition is good (condition 8). The underside of the deck is in good condition. The girders are in good condition with no signs of rust. The bearings appear to be well seated and in good condition.

D. Wingwalls and Abutments The overall condition is good (condition 7). The granite block wingwall on the southwest side has some cracked mortar with vegetation growth in the cracks.

The other wingwalls are in good

condition. The abutments are in good

condition.

E. Channel

The channel is in good condition (condition 8). There is heavy vegetation upstream and downstream.

CONDITION RATING

Previous in-depth: 7 Interim 1987: Interim 1989: 7 Routine 1991: Routine 1993:

RECOMMENDATIONS

Status of Previous Recommendations

There were no previous recommendations.

Revised Recommendations

Repair cracked mortar on southeast wingwall.

Estimated Cost

\$1,000

STRUCTURES INSPECTION FIELD REPORT

ROUTINE INSPECTION

THOMASTON	G		bridge dep	t. no.	8-structi	ure no.	006	90-date inspe 8 24 19	
2-dist. 104-highway sy	_ •	22-owner CENE	D	27-year ∪ Y	built المنبئ	106-year rebuilt		11-milepoint	
43-structure type	1 .01 .06 5	346 360		quality of	control en	gineer Nick	FOR	BES	
07-facility carried	N WIDE FU	ROAD		team le	eader	DSEPH C			
06-features intersected	PRTH FIELD	- RON O		team r	nembers	,			
NORTHE	ELD BROOK	>		μ.,	ICR10	M. DESC HEA	res i	٧٠٠٠٠	
item 58	8	item 59			3	item 60_ SUBSTRU	ICTURE	8]
DECK			RUCTURE		8	1. Abutme			
 Wearing Surface 	8		g Devices		8	a	-Wings	7	
2. Deck-Condition	8	2. Stringe 3. Diaphr			8	_	-Backwall		
Stay in Place Forms			s or Beams		8	ŀ	-Bridge S I-Breastwa		
4. Curbs	8	5. Floor I			8		-Footings		
5. Median	MA	6. Trusse			NA		-Piles	NA	
6. Sidewalks	NA	7. Rivets	or Bolts		8	1	j-Erosion	NA-	
7. Parapet	NA	8. Welds	i		NA		n-Settleme	ent 🕏	!
8. Railing	8	9. Collisi	on Damage		てき	2. Piers o	r Bents a-Caps	JA	1
9. Anti Missile Fence	DA.	10. Load	Deflection		8		o-Column	NA	-
10. Drains	8	11. Memb	er Alignmer	it	8		-Web	PA	
11. Lighting Standards	PA	12. Load	Vibration		8	1	d-Footing	NA	
12. Utilities	NA	13. Paint-	Ероху		8		e-Piles	<u>14</u>	
13. Deck Joints	7	14. Year I	Painted		91		f-Scour g-Settlem	አሉ ent	_
	<u> </u>	15. Under	r Clearance	ft	ir	3. Collisio	-	o to	
14. Approach Settlemen	III. Laffard	Clearan	ce Signs	y	es 🗶 no		_]
					1.6'	/- N l l A - l			
Actual Posting	H 3 352	Single NA		Overn	ead Signs yes	s (attached to brid	ge)		
Recommended Posting From Rating Book	NA NA NA	IIT		1. We					
SIGNS IN PLACE Y or N	at bridge	advai	_ 1	 Bol Co 	lts ndition				
LEGIBILITY	_			Item9	93b U/W	Inspection Date:		_	
ITEM 61-channel and	I channel protecti	on 7		36-	Traffic S	afety features			
nannel scour	ජී 5. rip	rap or slope p	aving Lie] , ,	oridge rai	lina	36 • A	condition	í
2. embankment erosi		rap or slope p ectiveness	В	2. t	ransition		N.C.	8	
3. fender system	7. del	oris	7] 3. a	approach	guardrail	L	8 8 8	
4. spur dikes & jetties	NA 8. veg	getation	7	J 4. 9	guardrail	terminal	AG	8	
X-IINKNOWN		NA-NO	LAPPIJO	ARLE			NACC	ESSIBLE.	Marin Production

PROJECT: BLACK LOCK LAKE

NAME: OLD NOATHFIELD LD BRIDGE

LOCATION: THOMASTON, CT

BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	No
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	No YES No No YES No
3. Are any characteristics of an aggressive stream or waterway present?	YES
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	No No No No No No
4. Is the bridge located on a stream reach with any adverse flow characteristics?	No
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	70 70 70 70 70 70 70
5. Other comments or observations.	No

HOP BROOK LAKE BRIDGE ON OLD ROUTE 63, MIDDLEBURY, CT FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 24 August 1993

DATE OF PREVIOUS INSPECTIONS: In-depth, Dec 84

Routine, Sept 87
Routine, Aug 89
Routine, Sept 91

RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	23T	32T	The 8 ton rating suggested in the 1984 in-depth inspection can be increased to the full inventory capacity since the deteriorated concrete of the arched section has been satisfactorally repaired.
3	38T	54T	
3S2	55T	77T	
3	61T	86T	

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway, Railings, and Deck.

The general condition is good (condition 7). The bituminous wearing surface on the north approach and south approach has some minor rutting. There are slight depressions at the transitions to the deck from the north and south approaches. The wearing surfaces on the north and south approaches have some minor rutting. Small stones from a chip seal have been left along the curb. The rails on the bridge deck have been recently patched are in good condition. The approach guardrails are in good condition.

B. Fascia and Curbs

The general condition is good (condition 8). The overall condition of the curbing is good. The curbs have recently been patched with concrete; however, the concrete has some minor surface deterioration. The fascias are in very good condition.

C. Underside of Deck and Bearings.

The overall condition is good (condition 7). The arched section has been recently repaired and has a new coating of "shot-crete". The coloring of the "shot-crete" is inconsistent and varies from very light gray to dark gray. The tee beams on the west side are in good condition with some minor honeycombing. The tee beams on the east side have a few spalls and minor honeycombing.

D. Wingwalls and Abutments

The overall condition is good (condition 7). The north and south abutments are in good condition. The weepholes on the south abutment are clear. The weepholes on the north abutment are buried by aggredation. The wingwalls are in good condition; however, there is miscellaneous vegetation growing in front on them.

E. Channel

The channel is in good condition (condition 7). The bridge is located on a bend in the river. This is causing aggredation along the northern abutment and creates the possibility of scour along the southern abutment. There is a confluence just west of the northern abutment.

CONDITION RATING

Previous in-depth: 7
Interim 1987: 7
Interim 1989: 5
Routine 1991: 5
Routine 1993: 7

RECOMMENDATIONS

Status of Previous Recommendations

Remove trees and vegetation in front of wingwalls.

Not done

Revised Recommendations

Implement the previous recommendation.

Total Estimated Cost

\$5000

STRUCTURES INSPECTION FIELD REPORT

ROUTINE INSPECTION

MIDDLEBURY, CT	bridge dep		e no. SEDCTOFICOT	90-date inspected 8 24 93
2-dist. 104-highway system	22-owner CENED	27-year built	106-year rebuilt 1944	11-milepoint
43-structure type		quality control engin	neer NICK FORBES	
07-facility carried	d CoNC. TEE	team leader		
OLD ROUTE 6	3 / LEC ANEA ACCESS		SEPH COLUCK 1	
06-features intersected Hof Blook	7450635	team members	M. Descheizs A	۸ معدد ۱۸
Hof Blook_				
item 58 DECK	item 59 SUPERSTRUCTURE		substructure 1. Abutments	7
1. Wearing Surface	1. Bearing Devices	NA	a-Wings	8
2. Deck-Condition	2. Stringers	7	b-Backwa	all MA
3. Stay in Place Forms	3. Diaphragms		c-Bridge	
4. Curbs	4. Girders or Beams 5. Floor Beams	NA	d-Breastv e-Footing	7.6
5. Median	6. Trusses	WA	f-Piles	IA
6. Sidewalks	7. Rivets or Bolts	NA	g-Erosior	
7. Parapet	8. Welds .	NA.	h-Settlerr 2. Piers or Bents	nent &
8. Railing	9. Collision Damage		a-Caps	No.
9. Anti Missile Fence	10. Load Deflection	8	b-Column	-
10. Drains	11. Member Alignme		c-Web	NA .
11. Lighting Standards	12. Load Vibration	8	d-Footing	JA AU
12. Utilities	13. Paint-Epoxy	NA NA	e-Piles f-Scour	
13. Deck Joints	14. Year Painted		g-Settlen	
14. Approach Settlement	15. Under Clearance		3. Collision Dama	
	Clearance Signs		4. Hydraulic-Adeq	uacy 7
	17 April	8		
Actual Posting H 3 3S2	Single	Overhead Signs (a	attached to bridge) no	
Recommended Posting From Rating Book	9	1. Welds		
SIGNS IN PLACE at bridge	advance	Bolts Condition		
Y or N	*	5. Condition	· · · · · · · · · · · · · · · · · · ·	
LEGIBILITY		Item93b U/W In	spection Date:	
ITEM 61-channel and channel protect	tion 7	36-Traffic Safe	•	condition
2. embankment erosion 3. fender system 6. ef	o rap or slope paving fectiveness 7 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2 2	1. bridge railin 2. transitions 3. approach gr 4. guardrail ter	uardrail	condition
X=UNKNOWN	NA=NOT APPLIC	CABLE	JA=INACC	ESSIBLE

PROJECT: Hoy Brook LAKE
NAME: OLD RT 63 BRIDGE
LOCATION: MIDDLEBURY CT

BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	No
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	Yes
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	NO YES NO NO HO YES NO
3. Are any characteristics of an aggressive stream or waterway present?	YES
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	YES NO NO NO
4. Is the bridge located on a stream reach with any adverse flow characteristics?	YES
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	У <u>е</u> S <u>No</u> <u>No</u> <u>No</u> <u>No</u>
5. Other comments or observations.	No

TULLY LAKE DOANE HILL ROAD BRIDGE, ROYALSTON, MA FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 24 June 93

<u>DATE OF PREVIOUS INSPECTIONS</u>: Inventory, 24 September 84 Routine, 15 September 87 Routine, 7 September 89 Routine, 11 July 91

RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	13.5T	25.4T	No change in ratings
3	16.0T	30.1T	-
3S2	24.7T	46.7T	
3-3	31.0T	57.6T	

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway and Railings: Overall condition 6. A New tar and gravel surface coat has recently been applied to the road and the bridge. When placing this coat, however, several of the vertical deck drains were covered and are now blocked. The new surface coating also continued across the joints in the deck. The new surfacing was not compacted well as it approached the openings in the curbings and therefore makes these openings ineffective for drainage. The extensive vegetation growth in the openings also creates an obstruction to the proper drainage of the deck. Weight limit signs were not present. The 3"x8" timber rails which are dried out and brittle are loose and inadequate. and are loose to the touch. The cable quard rails at the approaches to the deck are in good condition, however, they are very loose and need to be tightened and repaired.

B. Curbs & Fascias:

Overall condition 6. There is extensive spalling and wear on both curbs. drainage openings, as previously mentioned, are mostly filled with

vegetation and debris. There is extensive spalling and efflorescence along the exterior fascias of the bridge.

C. Underside of Deck:

Overall condition 7. Minor spalling around deck drains was noted. Most of the structural steel exhibited moderate rusting. The exterior beams show the greatest amount of rust. The beam on the interior of the north face of the bridge which has been noted as not having enough clearance, has not yet been cut. It is recommended that this beam be cut in order to allow 2" to 2 1/2" of clearance from the face of the abutment. The bearings are in good condition with minor rust and debris buildup.

D. Wingwalls/Abutments:

Overall condition 8. The wingwalls and abutments are in good condition. Bonding and alignment are good. The walls show no signs of distress.

E. Channel:

The overall condition is 8. The water flows smoothly through the channel with little or no debris buildup. Some minor abrasion was evident at the base of the abutments below the flow line.

CONDITION RATING

Inventory, 1984 7
Routine, 1987 7
Routine, 1989 7
Routine, 1991 7
Routine, 1993 7

RECOMMENDATIONS:

Status of Previous Recommendations

1. Repair loose guard rail cables on northeast N approach; repair detached upper guardrail cable on southwest approach; replace timber bridge rail with steel tubular section.

Estimated cost \$7000.

Not done

2. Clear debris from fascia openings and patch spalled areas with polymer modified repair mortar. Estimated cost \$3000.

Not done

3. Clean all debris and vegetation from gutters. Repair pavement on approaches and deck by cold

Not done

planing 1" from existing and repaving; clean deterioration from around drains; compact new material around drains prior to repaving.

Estimated cost \$5000.

4. Clean and paint all structural steel and bearings. Cut or burn web and bottom flange of first interior beam (North side, east abutment) as required to re-establish a minimum clearance of two inches.

Not done

Estimated cost \$15000.

Revised Recommendations

1. Repair loose guardrail cables on northeast approach; repair detached upper guardrail cable on southwest approach; replace timber bridge railing with new railing.

Estimated cost \$7000.

2. Clear debris from fascia openings and patch spalled areas with polymer modified repair mortar.

Estimated cost \$3000.

- 3. Clean all debris and vegetation from gutters. Can be done by project personnel.
- 4. Clean and paint all structural steel and bearings. Cut or burn web and bottom flange of first interior beam (North side, east abutment) as required to re-establish a minimum clearance of two inches.

Estimated cost \$15000.

Total Estimated Cost \$25000.

STRUCTURES INSPECTION FIELD REPORT

ROUTINE INSPECTION

Č.,				bridge dep	t. no.	8-structu	ıre no.		90-date ii	i l
120YAL			AKE	COE			EDMA25			193_
2-dist.	104-highway sy	ystem	22-owner		27-year i		106-year rel	ouilt	11-milepo	ı
	11 NOW FEDERAL COE				19:				0008	10
43-structure	• •	_			quality c	ontrol eng		_		
(<u></u> <u> </u>		ANDE BEAM	101 COV.	DECK	team lea		FOCRE	3		
Do Ar		POND				SEPH.	Comica	i- (
06-features					+	nembers				
Tille	y PIVER	_			H.	Desci	tenes	F. FUNG		
			i4 50				item 6	n		8
item 58	_	6	item 59	FOLICTURE			-	TRUCTURE		O
DECK				RUCTURE		7	1. Abi	utments		
1. We	aring Surface	<u>'1</u>	İ	g Devices		122		a-Wings		8
2. Dec	ck-Condition	7	2. String			7		b-Backwal	11	8
3. Sta	y in Place Forms	₩	3. Diaphi	_		7		c-Bridge S		<u>8</u>
4. Cur	•	6		s or Beams		414		d-Breastw		<u> </u>
5. Me		NA	5. Floor			h*		e-Footings f-Piles	5	PA PA
	ewalks	NA	6. Trusse			7		g-Erosion		3
7. Par		6		or Bolts		NA		h-Settlem		8
1	•	· ·	8. Welds	i on Damage		$\overline{\mathbf{X}}$	2. Pie	ers or Bents		
8. Rai	_	74	ļ	on Damage Deflection				a-Caps		5 V
	ti Missile Fence	[b]	/	per Alignmer	nt	8	·	b-Column c-Web		NV NV
10. Dra		h'h K	12. Load	•	it	×		d-Footing		NA
11. Lig	hting Standards		13. Paint-			6		e-Piles		PA
12. Util	lities	Aid	14. Year					f-Scour		NA
13. De	ck Joints	7	,	r Clearance	ft	NA in		g-Settlem	ent	NA
14. Ap	proach Settleme	nt 8		ce Signs		es NA no	3. Co	Ilision Damag	-	\boxtimes
	•		Clearan	ce oigns	ye	3 [7]	′ 4. Hy	draulic-Adeqı	uacy	8
Actual Po	sting	H 3 3S2	Single	e	Overh	ead Signs yes	s (attached to			
						,		- -		
Recomme	ended Posting			1	1. Wel	ds.	174		•	
From Rat	ing Book	13 16 25]			NA	<u> </u>		
0101:0::	N. D. AGE	- x 12 -1	•		2. Bolt	S				
SIGNS II	N PLACE	at bridge	adva [7]		3. Cor	dition	N	\mathbf{Y}		
I OI IN		2		_						
LEGIBIL	ITY	NA	N		Item90	3b U/W	Inspection Da	ate: Non	٤	
	C1 abortal s	d abannal mests at	on [007	Froffic C	ofatu factura	•		
I EWI	or-channer and	d channel protecti			30-	raille Sa	afety feature	<u>36</u>	cond	
	nnel scour		rap or slope p		- I	ridge rail		. 0	(
1	oankment eros		ectiveness	NF	٠. ٠.	ansitions				1
	der system Ir dikes & jetties	7. del 8. vec - 3	oris getation	3	-	pproach uardrail 1	guardrail terminal			
spu	- unes a jeules	5 <u>PF</u> 0. Ve(۰ ۱ ۲ ۰ 9	uai ui aii i				<u></u>
X=U	NKNOWN		NA=NO	LAPPLIC	CABLE		31.	A=INACC	ESSIBL	E

PROJECT: TULL! LAVE

NAME: DORNE HILL RIND

LOCATION: ROYALSTON MA.

BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	No
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	yes
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. 	9es 00
d. Designs which collect ice and debris.e. All water must pass through or over structure.f. Other.	
3. Are any characteristics of an aggressive stream or waterway present?	40
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. 	No No
c. Steep slopes.d. High velocities.e. Any history of highway or bridge damage during	<u> </u>
past floods. f. Other.	<u>—</u>
4. Is the bridge located on a stream reach with any adverse flow characteristics?	_ N°
a. Crossing near stream confluence.b. Crossing of tributary stream near confluence	No
with larger streams.	<u> </u>
c. Crossing on sharp bend in stream.	No
d. Location on alluvial fan.	0
e. Other.	<u>84</u>
5. Other comments or observations.	None

EVERETT LAKE CHOATE BROOK FY93 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION: 9 Sept 93

DATE OF PREVIOUS INSPECTIONS: 31 July 91 Routine

11 Sep 89 Routine 17 Sep 87 Routine 25 Mar 85 In-depth

RATING (T = TONS)

Type	Inventory	Operating	Comments
Н	2.0 T	4.4 T	Ratings from 1985
3	3.6 T	7.9 T	in-depth.
3S2	5.7 T	12.5 T	

Note: Ratings are estimated for H-20 loading for the new concrete deck for 1993 and final calculation will be performed within FY94.

EVALUATION (see attached field report)

A.	Approaches	Overall rating is 6. Guard rails are new but only 25' long on east side and no erosion control on both sides.
В.	Bridge Deck	Overall rating is 7. New bridge deck with guard rails on both sides. Missing bolts were located on the middle of the south guard rails. Most of the I-beams posts do not line up their centerline axis.

C. Substructure

Overall rating is 7. At the northeast abutment corner, a one and half foot deep scour is located. There are honey comb and hairline cracks at the southeast bridge abutment. Tree branches and debris are built up on the southside of the bridge deck.

CONDITION RATING:	Previous in-depth:	6
	Routine 1987:	6
	Routine 1989:	5
	Routine 1991:	4
	Routine 1993:	7

RECOMMENDATIONS:

Recommendations

1. The length of the guardrail for the eastside approach should be increased another 25 feet due to the sharp curve and deep drop at the edge.

Estimated cost: \$ 1500.00

2. There should be some erosion control on the embankments along both side approaches.

Estimated cost: \$ 2000.00

3. The project personnel should remove the tree branches and debris under or near the bridge deck.

Total estimated cost: \$ 3500.00

STRUCTURES INSPECTION FIELD REPORT

ROUTINE INSPECTION

C.1.y	/1	N.H		bridge dep	t. no.	8-structu	re no.	90-date inspected
2-dist.	WEARE 104-highway sy		22-owner	<u> </u>	27-year		106-year rebuilt	11-milepoint
		***********	CORPS 9	FENGR		20	1993	
43-structure	type CONCRETE	SLAB S	INGLE S	PAN	quality c	ontrol eng <i>Mには</i>		
07-facility ca	arried RECRA	EATION AR	EA ACCE	es Rd	team le	ader <i>JDE</i>	COLUCCI	
06-features	intersected				team m	nembers		
	<u> </u>	HOATE A	SRVD K	***	M. 2) ESCH!	ENES / F. Fu	N4
item 58		7	<u>item 59</u>			7	item 60	7
DECK		<u> </u>	SUPERST	RUCTURE			SUBSTRUCTURE	
1. Wea	aring Surface	8	1. Bearin	g Devices		12	1. Abutments	. 7
	ck-Condition	8	2. Ştringe	ers		NA	a-Wings b-Backw	ali 🖁
	y in Place Forms	MA	3. Diaphi	ragms		NA	c-Bridge	Seats 8
	•	8	4. Girder	s or Beams		NA	d-Breast	wall 7
4. Cur		MA	5. Floor i	Beams		8	e-Footing	
5. Med		₹A.	6. Trusse	es		NF	f-Piles	n 6
6. Side			7. Rivets	or Bolts		1	g-Erosio h-Settler	- 7.
7. Par		₩	8. Welds			NA	2. Piers or Bents	
8. Rai	ling	7	9. Collisi	on Damage		MA	a-Caps	MA
9. Ant	i Missile Fence	W#	10. Load	Deflection		<i>-144</i>	b-Colum	
10. Dra	ins	NA	i	er Alignmen	t	8	c-Web	7/A
11. Ligi	hting Standards	116	12. Load			NA	d-Footin	g 🚧
12. Util	ities	NA	13. Paint-	•			e-Piles f-Scour	M
13. Dec	ck Joints	NA	14. Year I		NA	<u>/⊻F</u>	g-Settler	
14. App	oroach Settlemer	nt NA		Clearance.	' 	in	3. Collision Dama	,
			Clearand	ce Signs	ye	es no	4. Hydraulic-Adeo	quacy VP
			1.00		Overhe	ad Signs	(attached to bridge)	
Actual Pos	sting	H 3 3S2	Single	•		yes	no no	
			N/A					
	ended Posting		20		1. Wel	ds	MA	
From Rati	ng book	ţ			2. Bolt	•	NA	
SIGNS IN	N PLACE	at bridge	advar	nce	2. BOIL	5	4/1	
Y or N		N	N	٦ ١	3. Con	dition	NA	
LECIDILI	TV	WA	M	- 1			9-10-10-10-10-10-10-10-10-10-10-10-10-10-	
LEGIBILI	T	1011	<u> </u>	.	Item93	Bb U/W I	nspection Date:	IONE
ITEM 6	31-channel and	channel protect	ion 🔼		36-T	raffic Sa	fety features	
	1 -	[Z] - ·					36	condition
1	nnel scour ankment erosic		rap or slope p ectiveness	eaving 6		ridge raili ansitions		
1	ler system	//₄ 7. de		7		ansilions pproach (] NA
	dikes & jetties		getation	7		uardrail te		NA.
MAY ATT	IKNOWN		LON-AN-	APPLIC	ARIE		IA-INACO	PESSIRI E

PROJECT: EVERETT LAKE

NAME: CHOATE BROOK

LOCATION: WEARE, N.H.

BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	YES
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other. 	YES YES NO YES NO
3. Are any characteristics of an aggressive stream or waterway present?	YES
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	NO YES YES UNKNOWN
4. Is the bridge located on a stream reach with any adverse flow characteristics?	
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	No No No No
5. Other comments or observations.	NONE

OTTER BROOK LAKE EXIT BRIDGE, KEENE, N.H. FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 18 August 1993

DATE OF PREVIOUS INSPECTIONS: In-depth, *

Routine, Sept 87 Routine, Sept 89

Routine, 22 August 1991

RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	18.0T	32.6T	Load capacities
3	22.1T	39.9T	recalculated for
3S2	34.4T	62.1T	prestressed beams

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway, Railings, and Deck.

The overall condition is good (condition 7). The bituminous wearing surface onthe deck is in good condition. The south approach surface is in good condition with a minor crack at the transition to the bridge deck. The north approach has some minor rutting along wheel lines and a crack at the transition to the bridge deck. The terminal unit of the guardrail in the northwest corner is damaged. The design of the existing terminal unit in this location is poor and should be extended around the corner and buried. The southeast top railing is loose.

B. Fascia and Curbs

The overall condition is good (condition 7). Both the curbs and fascias have hairline cracks approximately every two feet. There is also some spalling at the caps covering the transrverse posttensioned reinforcing. Minor debris and vegetation is collecting along the curbing.

C. Underside of Deck and Bearings.

The overall condition is good (condition 7). The underside of the deck is in good condition. There is some minor leakage of water from the deck onto the south abutment. No problems were noted with the bearings.

with the bearings.

D.	Wingwalls	The	overall	condition	is
	and Abutments	6).	The nort	th abutment	t is

6). The north abutment is in good condition. The northeast footing has a spall measuring two foot by two foot by six inches. There is also evidence of scour and erosion along the northeast

wingwall.

E. Channel The overall rating is 5. The water is

deepest along the abutments. The north

east abutment is scoured and

deteriorated as noted on previous reports. The channel contains many rocks and has the potential to collect

debris.

<u>CONDITION RATING</u> Interim 1987: 7

Interim 1989: 6 Routine 1991: 6 Routine 1993: 6

RECOMMENDATIONS

Status of Previous Recommendations

Cost Est Status

fair (condition

1.	Repair erosion and deteriorated	\$20,000	Not Done
	concrete at the base of the abutments.		

2.	Provide stone apron at abutment	\$15,000	Not Done
	as scour remedial action.		

3.	Remove vegetation	from wingwalls	\$500	Not Done
	and curbs.			

Total \$35,500

Revised Recommendations

Implement above recommendations.

Extend and bury northeast \$1500 guardrail terminal unit.

Total Updated Estimated Cost \$37,000

STRUCTURES INSPECTION FIELD REPORT

ROUTINE INSPECTION

EENE NH			bridge dep	t. no.	8-structu	ure no. "NEDNH 331000	_	0-date inspected
2-dist. 104-highway syst	em	22-owner CoE		27-year		106-year rebuilt		-milepoint
43-structure type PRESTRESSED Co	KRETE B		BEAMS		ontrol en			
07-facility carried				team le				
	XIT				した	COLUCT_		
06-features intersected				team m	nembers			
					Eut	n locio m	iark 1	DESCHENES
item 58	7	item 59			7	item 60	***	4
DECK		SUPERST	RUCTURE			SUBSTRUCT	URE	(*
1. Wearing Surface	7	1. Bearin	g Devices		7	1. Abutments		7
Deck-Condition	7	2. Stringe	ers		NA	a-Wii	ngs ckwall	G ·
Stay in Place Forms	NA	3. Diaphr	ragms		NA	·	dge Seat	
4. Curbs	7	4. Girder	s or Beams		7		eastwall	7
5. Median	NA	5. Floor 8	Beams		NN		otings	۵
6. Sidewalks	N	6. Trusse	es		NA	f-Pile	_	NA
	1	7. Rivets			NA	g-Erd	ttlement	NA
7. Parapet	7	8. Welds			A lea	2. Piers or Be		
8. Railing	NA		on Damage		8	a-Ca	ps	NA
9. Anti Missile Fence	JA.	10. Load [8	b-Co	lumn	مولي
10. Drains			er Alignment		8	c-We		
11. Lighting Standards	NA	12. Load \			A lea	d-Fo	•	ra Na
12. Utilities	Nh	13. Paint-l	•		NA	f-Sc		
13. Deck Joints	7		Clearance _	44	in	g-Se	ttlement	
14. Approach Settlement	7	Clearance				3. Collision Da	amage	B
·		Clearanc	e Signs	ye	s 🔀 no	4. Hydraulic-A	Adequacy	7
Actual Posting	H 3 3S2	Single		Overhe	ad Signs	(attached to bridge)		
_	IS NA NA	IA.	·		yes	X no		
Recommended Posting						NA		
From Rating Book	NA PIN PIN	NA		1. Welc	is			
CIONO IN DI ACE				2. Bolts	5	NW		
SIGNS IN PLACE at	bridge	advan		3. Cond	dition	NA		
	<u>¥</u>			0. 00				
LEGIBILITY	у	NA		Item93	b U/W II	nspection Date:	NONE	•
'TEM 61-channel and ch	annel protection	on [/_		36-T	raffic Saf	ety features		
	P		. –	55-11	ano oai	or, realties	<u>36</u>	con <u>diti</u> on
1 annel scour 2. embankment erosion		rap or slope pa ctiveness	aving HA		dge railir	ng	14	7
3. fender system	7. deb		1	1	insitions proach g	uardrail	N.	7
4. spur dikes & jetties		etation	7		ardrail te		NW	7

NA-NOT APPLICABLE

JA-INACCESSIBLE

X=UNKNOWN

PROJECT:_			
NAME:	EXIT	BRIDGE	
LOCATION:	KEEN	HE MI	H

BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	YES
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non- 	N/A
redundant support systems. c. Inadequate waterway openings.	XES No
d. Designs which collect ice and debris.e. All water must pass through or over structure.	YES No
f. Other.	N/A
3. Are any characteristics of an aggressive stream or waterway present?	YES
a. Active degradation or aggredation of streambed.b. Significant lateral movement or erosion of	YES
streambanks. c. Steep slopes.	No
d. High velocities.e. Any history of highway or bridge damage during past floods.	No No
f. Other.	N/A
4. Is the bridge located on a stream reach with any adverse flow characteristics?	No
a. Crossing near stream confluence.	No
b. Crossing of tributary stream near confluence with larger streams.c. Crossing on sharp bend in stream.	NO NO
d. Location on alluvial fan. e. Other.	No N/A
5. Other comments or observations.	No
venimente en energy volution	

NEW ENGLAND DIVISION

PAGE	
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LI Sepi 43				NEERS, U.S. AF	CM T	
SUBJECT PATIFIC	roc-	arree.	28006	EXIT	BRI DEF	
COMPUTATION		1111		i .		
COMPUTED BY A S		ı	CHECKED BY		DATE	3/2/193
COMPOSED OF The Party of the Pa			- CHECKED BI		UNIC -	

INJENTORY LOND PAR			
	HIS HIS	3	352
36x21. FLEXURE	23.0	28.2	43.9
SHEAR	49.3	58.5	106,5
42×21 FLEXULE	18.0	22.1	34.4
5-6+2	37.8	44.9	81.5
CHERATING LOOP PATIN	6		
26 K21 FLEWORE	45.5	55.8	86.9
SEA	82	97.6	177.5
48 VILL FLEW JAKE	32.6	39.9	62.1
	63.0	74.6	134.6

OVERALL HTING	HIS	3	352
TAJENTORY	18.0	22.1	34.4
OPERATING	32.6	39,9	62.1

27 Sept 49

CORPS OF ENGINEERS, U.S. ARMY SUBJECT OTTER BROOK LAKE ENTRANE EXT BRIDGES

× ZI" IN HC SLAPS CASE A

_ CHECKED BY ____ M.D

BRIDGE REPLACED 1987

SIMPLE SPAN, PRESTRESSED DECK BEAMS

LENGTH OF SPAN : 43 PEST

TYPE IV-36 (36" x 21" IN HC SLAB)

A = 529.80 in 2

I = 25747 in +

W = 551.9 #/FT

5x = I/C C = 21 /2

= $\frac{25747 \text{ in}^2}{21 \text{ in } /2}$ = $\frac{2452.1 \text{ in}^3}{2}$

INFORMATION GATHERED FROM TRANSMITTAL NO. 3230-002 RESUBMITTAL 9 OCTOBER 1985

fi'= 5000 Psi

feil = 4000 151

density = 150 PCF

STRANDS

12 - 1/2" \$ 270 K SELV STRANDS

STRAND AREA = 0.1530 in / STEAND

As= 0.1530 x 12 = 1.8360 in = Ast = 0.306 in Ast = 1.53in2

CG OF STRAND = 2" FROM BOTTOM OF BEAM / 3" FROM TOP

Ps= h - CG = 21" - 2" - 8.5" et = 7.5"

CORPS OF ENGINEERS, U.S. ARMY

SUBJECT	U TEUSTOOF	- LA-LG			
COMPUTATION	CASE A			1 1	
	me 1		41 1	B/ 19/92	

INITIAL TENSION ON STRAND = 28.92 KIPS = Pi PB = 28.92 x 10 = 289.2 KIPS (10 STRANDS) PT = 28.92 x 2 = 57.8 KIPS (2 STRANDS) INITIAL STRESS ON STRAND Finifial = Pa / As Finifial = 28.92 / 0.1530

DESIGN LOADS

DL SELF WT

551.9 #/FF

SUPPLIMENTAL DEAD LOADS

DL BIT TOPPING = (2.5") (150 /121/(3) = 93.75 = /FT LL HIS TRUCK

MDL = (0.5519 E/FT) (43FT) = 127.56 FT- K

MSDL = (0.09375 KIFT) (43 FT) = 21.67 FT-K

27 Sept 49 CORPS OF ENGINEERS, U.S. ARMY

SUBJECT OTTH BROOK LAKE

CASE A

COMPUTED BY MEI CHECKED BY M.D DATE 81993

LOSS OF PRESTRESS, AASHTO TABLE 9.16.2.2.

PRETENSIONED STRAND fe'= 5000 PSI 45,000 PSI

(ESTIMATED CALCULATION)

Formal = 189 - 45 = 144 KSI

Pfinal = 144 ksi (1.836 in²) = 264.38 kips = effective prestress

MP/S = MOMIENT DUE TO PRESTRESS = Pe = (264.38 EIPS) (8.5 in/12) = 187.27 FT-16.1PS

CALCULATED LOSS OF PRESTRESS

Fair = CONCRETE STRESS AT COT OF PRESTRESSING STEEL DUE
TO PRESTREGGING FONCE AND DEAD LOND OF BEAM
IMMEDIATELY AFTER TRANSFER

Fols = CONCRETE STRESS AT CG OF PRESTRESSING STEEL DUE
TO ALL DEAD LOADS EXCEPT THE DEAD LOADS PRESENT
AT THE TIME OF PRESTRESSING

 $f_{cir} = \frac{P_o}{A} + \left(\frac{P_oe^2}{T}\right)_b - \frac{M_1c}{T} - \left(\frac{P_oe^2}{T}\right)_T$ $= \frac{0.9(347,040)}{529.8} + \frac{0.9(289,260)(8.5)^2}{25747} - \frac{0.9(57800)}{25747} \frac{(127560)(12)8}{75747}$ = 589.54 + 730.39 - 113.65 - 505.34 = 700.94

 $f_{eds} = (21,670)(12)(7.5) = 75.75$

USE COMPOTER GENERATER Foir = 663

CALCULATED BY DISSIDER 5-6-85

27 Sept 49

CORPS OF ENGINEERS, U.S. ARMY

OTTEL BROOK LAKE

ME!

Chest &

COMPUTED BY -

COMPUTATION .

CHECKED BY __

LOSS OF PRESTLESS

Afs = SH + ES + CRc + CRs

SH= 17000 - 150 RH = 17000 - 150 (70) =

6500 (6.4)

RH = 70 (RELATIVE HUMIDITY)

ES = Es fair = 28000 (663) =

481-2 (6-9)

CRc = 12 (fair) - 7 (faps) = 12 (663) . 7 (59) = 7543 (9-9)

CR3 = 20,000 - 0.4 ES - 0.2 (SH + CR.)

= 20,000 - 0.4 (4842) - 0.2 (6500 + 7543) = 15,255 (9-1.

find = 189.02-34.14 = 154.88 KG1

Pfinel = 154.88 (1.836 in2) = 284.36 KIPS

Mels = MOMENT DUE 10 PRESTLESS = Pe

= (284.36 kip) (8.5 in/12) = 201.42

PT = (154.88 KG) (2) (0.1530) = 47.39 KIPS

PB = (154.88)(10)(0.1530) = 236.97 KIPS

MP/s = (47.39) (7.5)/12 = 29.62 FT-EIPS

MMs = (236.97)(8.5)/12 = 167.85 FT- KIPS

CORPS OF ENGINEERS. U.S. ARMY

SUBJECT ___

OTTEL PROPE LAKE

CASE

CHECKED BY _____ M.D

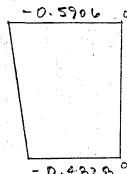
8/19/73

STRESSES

=
$$\frac{284.36}{529.8} \pm \frac{29.62(12)}{2452.1} \pm \frac{167.85(12)}{2452.1} \pm \frac{149.23(12)}{2452.1}$$

ALLOWARLE STRESSES

STREET AVAILABLE FOR LIVE LOAD



CORPS OF ENGINEERS, U.S. ARMY brook three

OTTEL SUBJECT __

CASE

MEL COMPUTED BY _

OPERATING MAINIAL 5.416

FOR PRESTRESSED CONCRETE MEMBERS, THE REINIFORCEMENT INDEX DETERMINED BY AASHTO 9.18.1 DOES NOT EXCEED 0.30, THE OPERATING RATING SHALL RESULT IN MOMENTS NOT TO EXCEED 0.75 THE ULTIMATE MOMENT CAPACITY AASHTO 9.17

As = 10(0.1530 in 2) = 1.530 in 2

d = h - cg of lower strands = 21" - 2" = 19"

fe' = 5.0 ks1

fs' = 270 KS1

P* - 4= 1 bd = 1.530 /(36)(19) = 0.0022

P* - BATIO OF PRESTRESSING STEEL

STEEL STRESSES (BONDED MEMBERS) AASHTO 9,17,4,1 It'sus any stress in prestressing steel at ultimate load f*su = fs' (1-0.5 fs' p*) = 270 KSI [1-0.5 (0.0022) 270 KSI) f*su = 253 69 kg1

REILIFORCEMENT INDEX (RECTALLAULAR SECTIONS) AASHOO 9.18.1 P* fsu = 0.0022 (253.69 5 KSI) = 0.1116

0.1116 L 0.30 OK

PAGE 7/22

27 Sept 49 CORPS OF ENGINEERS, U.S. ARMY

SUBJECT	100	10 1	-4 EK	-	Brase	استاسا	ሩ

COMPUTATION CASE A

COMPUTED BY ME 1 CHECKED BY M.D DATE \$ 19/29

CHECK LOCATION OF NEUTRAL AXIS

$$a = \frac{4s^{*} + su^{*}}{0.85 + 6'b} = \frac{(1.53)(253.69)}{0.85 + (5.0)(36)} = 2.54 \text{ in}$$

$$C = a/B$$
, = 2.54/0.80 = 3.17 in

FLEXURAL STRENGTH

$$M_n = A_5^* f_5^* \mathcal{L} \left(1 - 0.6 p^* f_5^* / f_c^* \right)$$

= $(1.53)(253.69)(19/12)[1 - 0.6 (0.116)] = 573.41 Fr- KIPS$

OPERATING CAPACITY AVAILABLE FOR LIVE COAD

CORPS OF ENGINEERS, U.S. ARMY

SUBJECT _

CAME

MEI

M.D

8/19/98

LIVE LOND

LOAD FRACTION S/D AAGHTO 3.23 4.3

S= WIDTH OF PRECAST MICHBER

C = K (W/L)

C - STIFFNESS PARAMETUR

W = OVERALL WIDTH OF BRIDGE (A)

L = SPAN LENGTH

C= 0.8 (14 FT/ 43FT) = 0.7605

C 5 5

D= (5.75 - 0.5 NL) + 0.7 NL (1-0,2 C) 1/2 (3-12)

NL = NUMBER OF LANES =

D= (5.75 - 0.5 (1)) + 0.7(1) (1-0.2 (0.2605)) 1/2-

D = 5.93

LORD FRACTION

5.93

 $\frac{1}{1} \frac{1}{1} \frac{1}$

ANSHTO 3.8.2.

LIVE LOADS FROM MANUAL PLATE Z

H20: 20/15 (140.95) (1.2976) (0.5058) = 123.35 FT-KIPS

H15: 140.95 (1,2976) (0,5050) = 92.51 FI FIFS

191.75 (1.2976) (0.5055) = 125.85 FT KIPS

352: 177 35 (1.2976) (015053) 116.40 FT 21PS

NED	FORM	223
27 9	ant 4	a

NEW ENGLAND DIVISION

CORPS OF ENGINEERS, U.S. ARMY

	0/	
1.	7/27	
PAGE	/	

SUBJECT		07	TER	Brook.
		6	Ner	A -

COMPUTATION ______

COMPUTED BY _____ CHECKED BY ____

MD

DATE 3/19/93

MOMENT PATING

TYPE	INVENDORY (TONS)	OPERATING (TONS)
H20	(141.9B)(207) / (23.35 = 23.02	(200.83)(20T)/23.35 = 45.5
H15	(141.98)(157)/92.51 = 23.02	(280.83)(15) 92.51 = 45.5
3	(141.98) (25)/ 125.85 = 28.20	(200 83)(25)/125.65 - 55.7
11		(250.83)36)/116.40 = 86.8

CORPS OF ENGINEERS, U.S. ARMY

SUBJECT _____

OTTEL BROOK LAKE

COMPUTATION _ CASE A

COMPUTED BY MEI

CHECKED BY W, C

DATE 8/19/93

CHECK SHEAR + RENFORCEMENT

CHECK MINIMUM REINFORCEMENT

 $f_r = 7.5\sqrt{f_c'} = 7.5\sqrt{5000} = 530.33$ (AASHTO 9.15.2.3) $Mer = f_r S_x = 0.53033(2452.1)/12 = 108.36$ 12 Mer = 1.2(108.36) = 130.04 pt - kips < 573.41 = MulOF

AASHTO 9.18.2

CHECK SHEAR #4 (GRASE 60) @ 15" OC

USE AASHTO 9.20 - THE USE OF 1979 INTERIM
METHOD IS ACCEPTABLE.

MAX SPACINO = 3/4 h = .75(21) = 15.75" 215" DE

 $Av = (\frac{V_u - V_c}{2})^{\frac{1}{2}}$ $f_{5y} = (0,000)^{\frac{1}{2}}$ $f_{5y} = (0,000)^{\frac{1}{2}}$ $f_{5y} = (0,000)^{\frac{1}{2}}$ $f_{5y} = (0,000)^{\frac{1}{2}}$

 $V_{u} - V_{e} = 2 \underbrace{Av f_{ey} jd}_{e} = 2(0.20)(60)(0.9)(19) = 27.36$

Ve=0.06fébyd 3 use smaller b'= 36-2(12)=12 Ve-180 b'jd)

Ve = 0.00 (5000) (12") (0.9) (19") = 61.56 KIPS Ve = 180 (12") (0.9) (19) = 36.94 KIPS CONTROLS

Vu - Ve = 27.36) Vu = 27.36 + 36.74 = 64.30 KIPS

1 Vz = 0.9 (64.30 ×) = 57.87 KIPS AASHTO 9.14

27 Sept 49 -

OTTELPROOF

CASE A

MEI COMPUTED BY __

As PER 1979 INTERIN ; EVERE SHEAR AT 1/4 PT

DL & SDL SHEKE AT & POINT

VDL +SDL = (0.5519 Elf + 0.09375 Elf) (43 - 43) = 6.94 K

LIVE LOAD SHEAR AT /4 POINT L= 43, 0.75L= 32.25, 0.25L= 10.7

IMPACT

impact = 50/125 + 32.23 = 0.3180 : I = 1.32 CLOSE FAOGEN MANNIN PLATE 64 7

H20: V= 20(32.25-2.8)(1.32)(0.5058) = 9.15 × 90

3: V= 25 (37.25 - 7.44) (.52) (0.5058) = 9 62 < 95

352: V= 36 (32.25 - 18.61) (1.32) (2.5068) = 762 6 75 MD

USE LOAD FACTOR METHOD

1.3 (VDL + 5/3 VLL + I) = + Vn = 57.57 CIPS 1, 5/3 VLL+= = QVL - VDL = 57.87 - 6.94 = 37.58 E

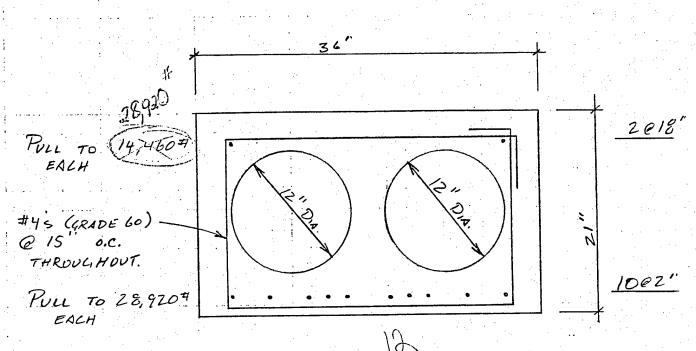
(Wellowy: 3/5 (37.50) = 22.55 EIPS

OFFERTING: 37.58

TYPE INVENTORY (TONES) OPELLY WITH (TONG) H15 (27.55)(20) /9,15 = 49.28 (37.58)(20) /9.15 = 82.14 3 (22.55 (25) /9.65 = 58.54 (31.50 (25) /9.63 97.56 3 52 (7.2.55)(36) /7.62 = 98 106.54 (31.58)(36)/7.62 = 177.54

8-6-85	DESIGN KLT		LEAP ASSOCIATES INTERNATIONAL INC.						SHEET 2
REVISED	CHECK	JOB				FOR			198 NO. PRC 513

CASE "A" 36" x ZI" HC SLAB



fé = 5000 PSI féi = 4000 PSI DEN = 150 PCF USE 70- 1/2" \$ Z70K SPLV STRANDS

27 Sept 49

CORPS OF ENGINEERS, U.S. ARMY

SUBJECT OTTERBROOK LAYS ENTTEANEC/EXIT BRIDGES

CASE "B" 48 x21' IN HE SLAB

MEI M.D ____ CHECKED BY __

BRIDGE REPLACED 1987

SIMPLE SPAN , PRESTRESSED DECL BEAMS

LENGTH OF SPAN : 43 FT

TYPE N-48 (36" x 21" IN HC SLAR)

fe' = 5000 PSI A= 703.26 in2

fai = 4000 psi I= 34517 in 4

w = 732. 6 plf density = 150 pef

5x = 34517m/21/3 = 3287.33 in =

FROM TRANSMITTAL NO 3230 - 002 9 OCTOBER 1993

STRAHDS

12 - 1/2" & 270 E SELV STEANIDS

Asx = 0.1530 x 12 = 1.8260 in 2

As* - = 0.306 m2

· As* B = 1,530 in 2

CA OF STRAND! Z" FRAM BOTTOM OF BEAM 3" FRM TOP OF BEAM

es = h - cq = 21" -2" - 8.5"

et = 7.5"

27 Sept 49

CORPS OF ENGINEERS, U.S. ARMY OFFICE BROOK

CASE

1 55 8

CHECKED BY -

INITIAL TENSION ON STRAND = P.

Pi = 2892 Kits

PB = 28.92 × 10 = 289.2 KIPS (10 STEANOS)

PT = 28.92 X 2 = 57.8 KIPS (2 STRAND)

INITIAL STRESS ON EX. STRAND

28.92 /0.1580 = 189.02

DESIGN LOADS

DEAD LONGS

DL SELF WY

732.6 plf

SUPPLEMENTAL DEAD LOADS

DL BITUMINOUS SURFACE CURB

(2.5") (150 pl+)/12 in/c (3') = 93.75 plf. (132 in2)/144 in2/42 (150 plf)=137.5 plf 231.25

LIVE LOND

HIS TRUCK

MOL = (0.7326)(43)2/8 = 169.32 FT-KIPS

MODED=(0.09375)(432)/8 = 21.67 FT-KIPS

MSUDO (0.1375) (43)2/B = 31.78 +T-KIPS

27 Sept 49 CORPS OF ENGINEERS, U.S. ARMY Oriva Epont

SUBJECT ____ CARE

MEI COMPUTED BY ___

M.D

LOSS OF PRESTEESS AASHTO THOLE 9.16, 2, 2

PRETENSIONED STRAND

fc' = 5000 psi

for = 415 (from design calculation 3/6/85)

fede = 103 ("

Dfc - SH + ES + CRc + CRc

5H = 17000 - 150 PH = 17000 - 150(70) = 6500 (6-4)

RH = 70

ES = (Es) feir = (28000) 415 = 303 1 (6-9)

CRc = 12(fer) - 7 (feds) = 12(415) - 7(108) = 4224 (9-9)

CRs = 20000 - 0.4 1ES - 0.2 (SH+CRe)

= 20000 - 0.4 (3031) - 0.2 (65001 424) =

30,398

frimal = 189.02 - 30.40 = 158.62 EGI

· Pfinal = 158.62 (1.836) = 291.23 KIPS

M1/6 - Monitaly DUE TO PRESTRESS : Pe

= 291,23 (8.5/12) = 206.28 FT-16185

PT = (158.62)(2)(.1530) = 48.54 KIPS

PB = (158.62)(10)(0.1530) = 242.69 FIPS

M 0/5x= (48.54) (7.5/12) = 30.34 FT. KIPS

M1/5 B = 242.62 (8.5/12) = 171.90 FT- KIPS

CORPS OF ENGINEERS, U.S. ARMY

DITEX BRODE CARE

CASE B

COMPUTED BY _____ME /

M.D

DATE 8/28/95

STRESSES

=
$$-\frac{291.23}{703.26}$$
 $\pm \frac{30.34^{'12}}{3287.33}$ $\pm \frac{171.90(12)\pm (169.32 + 21.67 + 31.78)(12)}{3287.33}$

ALLOWABLE SEESES

INVENTORY CAMPRICESION PIETE = 2.0 EST

6 Fe' IS RER'D FOR CAPACITY SEE AASHTO 9.15.2.2

STRESS AVAILABLE FOR LIVE LOATS

Top of Beam 2.0 -0.7106 = 1.2894

BOT OF BEAM 0.424 + 0.176 = 0.5419 (CONTROLS)

INVENTORY

MCAP = (0.5419)(3287.33)/12 = 148.45

27 Sept 49

CORPS OF ENGINEERS, U.S. ARMY OTTER BROOK LAKE

CASE B

COMPUTED BY

WE!

OPERATING MANJUAL 5.46

FOL PRESTRESSED CONCRETE MEMBERS, THE PENFORCEMENT INDEX DETERMINED BY AASHTO 9:18.1 DOES NOT EXCEED 0.30 , THE OFERATING FATING SHALL RESULT IN MOMENTS NOT TO EXCEED 0.75 THE ULTIMATE MOMENT CAPACITY AASHTO 9,17.

Ac* = 1.530 in2

d = h - cg of lower strands = 21"-2" = 19"

fe' = 5.0 KS1

fo' = 270 K51

pt = Ast/bd = 1.53 /48)(19) = 0.0017

P# = LATTO OF FRESTEESSING STEEL

STEEL STRESSES (BONDED MEMBERS) AASHTO 9. 17.4. 1

fru = any stress in prestressing steel at ult. land

 $f_{su} = f_{s'} \left(1 - 0.5 f_{s'} p^* = 270 \left[1 - 0.5 \left(0.0017 \right) \left(270 \right) \right]$

= 257.61 KS1

REINFORCEMENT INDEX (RECTANGULAR SECTIONS) ARSETO 9.18.1

P# fou = 0.0017. (253.69 KSI/5 KSI) = 0.0863

0.0863 6 0.30

NEW ENGLAND DIVISION

CORPS OF ENGINEERS, U.S. ARMY

OFTER BROOK CARCE

CHOR

COMPUTED BY MEI

8/23/23

CHECK LOCATION OF NEUTRAL AXIS $a = \frac{A_5 * A_5 u^*}{0.85 (5.0)(48)} = 1.90 \text{ in}$

B, = 0.80 AASHTO B.16.2.7

c = a/B, = 1.90 b.8 = 2375

2.375 6 4.5 OK TO USE RECTANGULAR AREA

FLEXURAL STRENGTH

Mu = As + for & (1 - 0.6 p* for /fe' = (1.53) (257.61) (21)/2 [1-0.6 (0.0369)]

= 654.03 - * NOTE: MU = OM n => O = 1.0 ONLY FOR FACTORY PRODUCED PRESTRESSED - M.D. AASHTO 9. H.

OPERATING: MEAP = 0.75 (Mu) = 0.75 (654.03) = 490 53 A.

OPERATING CAPACITY KVAILABLE FOR LIVE LOND · Moper = Menp - (MOL + Mach) = 490,53 - (222,77) = 267,76

CORPS OF ENGINEERS, U.S. ARMY

SUBJECT Office Elegate LARE

COMPUTATION _____ CASE &

COMPUTED BY

CKED BY ______M.D.

_ DATE _ 9/29/7

LIVE LOAD

Cale (w/L)

SID= LOAD FRACTION

S= WIDTH OF PRECENTAGER

C = STIFFAME PARRAMETER

WE OVERTLE BRIDGE WIDTH

L = SPAN LENGTH

K = 0.8

C= 0.8 (14/43) = 0.2605

c = 5

." D = (5.75 - 0.5 NL) + 0.7 NL(1-0.2 C) 1/2 (3-12) NL= # OF LANES

D= (5.75 - 0.5) + 0.7[1-0.2(0,2605)] = 5.93

S/D = 4/5.93 = 0.6745

IMPACT

INPACT: 50/(L+125) = 50/(45+125) = 0.2976

1. I 1.2976

LIVE LOADS FROM MANUAL

PLATE Z

. HZO: 20/15 (140.75) (112976) (0,6745) = 164.48 FT- KIPS

H15: (140.95) (1,2976) (0.6745) = 123.36 FT- KIPS

3: (111.75) (1.2976) (0.6745) = 157.83 FT- EIPS

3521 (177.35) (1.2976) (1.6745) = 155.22 = 1- EIFS

NEW ENGLAND DIVISION CORPS OF ENGINEERS, U.S. ARMY

 $\frac{20}{23}$

SUBJECT 772

Trib Brook LAKE

COMPUTATION _ CASE &

COMPUTED BY ME /

CHECKED BY -

M.D.

DATE 8/28/43

MOMENT RATING

TYPE	INVENTORY (TONS)	OPERATING (TOMS)	
	(148.45) (20T)/164.48 = 18.05		14 - 14 - 14 - 14 - 14 - 14 - 14 - 14 -
415	(148.45) (157)/123.36 = 18.05	(267.76)(157)/(12336) =	32.56
3	(148.45) (257)/ 167.83= 22.11	(247.76)(257)/167.83 =	39.89
352	(48.45)(36T)/ 155.22= 34.43	(148-45 (36T)/155.22 =	34.45

48" FLANK CONTROLS PATING - M.D.

MEI

M.D.

CHECK ENERGY & PEINFORCEMENT

fr= 7.5 \fe' = 75\5000 = 530.33 (AASHTO 9.15.2.3)

Mer = f 1x = 0.53033 (3287.33)/12 = 145.28 FT- KIPS

1.2 Mcr = 1.2 (145.28) = 174.34 FT- KIPS < 654.03 OF

AASHTO 9.18.2

CHECK SHEAR #4 (GR 60) @ 15" or

USE AASHTO 9.20, 1979 INTERIM METMOD IS ACCEPTABLE

MAY SPACHIG = 3/1. h = 0.75 (21) = 15.75 2 15 01

Av = (Yu-Ve)s fry: 60,000 ps: Assume j = 0.9

Vu- Ve = ZAV fsyjd = Z(0.20)(60)(0.9)(19) = 27.36

Ver o. o. fe b'jol ? use smaller Ve = 180 bij d

6'= 48-2(12)-10

Vc = 0.06 (5000)(14")(0.9)(19) = 71.82 x1fs

· Vc = 180 (14) (019) (19) = 43.09 KIPS

Val = Ve : 27.36 EIPS Vu = 27.36 + 42.09 = 70.45 MPS

ØVe = 0.9 (70.45 €) = 63.41 EIES

AAGHTO 9.14

CORPS OF ENGINEERS, U.S. ARMY

MEL

M.D.

AS PER 1979 INTERIM; CHECK SULAR AT 1/2 FT

DL + SDL STUX AT VA PT

VOL+SOL = (0,732.6+.231.25) (43 -43) = 10.36 EIFS

LIVE LOAD SHOPE AT VA FOINT

0.75 L = 32.25 0.25 - 0.75

MPRET = 50/125 + 52.20 = 0.2130 M. I = 1.32

No minumen 130 mi

MALLIA PLATE 6 & 7

420: V= 20(32:35-53 (1.32)(0.6745)= 12.20 k

V= 25 (32.25 -7.01) (182 0.6715)= 12.84 K

V= 36 (32.25.18.6) (1.32) (0.674) = 10.17 12

DE LOND FACTOR METHOD

3 (VDL + 53 VLL +1) = & VLL = 63.41 KIPS

11 5/3 Vuri : 6/4 - Vol = 63.4 - 10.36 = 33.42 6/15

INVEHTORY: 3/5 (38.42) = 23.05

OF 11 1 38.43

West Topy TYPE (23,05)(20)/12,20 = 37.79 3 (25,05) (25) / 2.84 = 44.83 (38.42) (25) / 12.84 = 74.81 252 1 (22,05) (34) / 10, 7 = 81.54

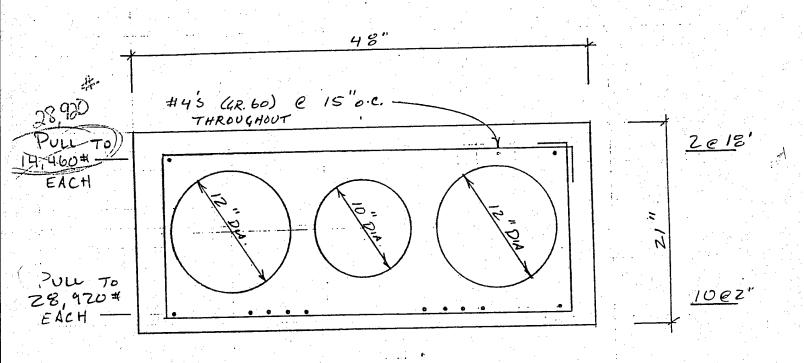
(3842) (20) / 12.20 = 62.98

oferation

(38.42 (36) / 10.17 1 136.00

8-7-85	KLT	LEAP ASSOCIATES INTERNA	
SED	CHECK	OB FOR	JOB NO. PR & S13

CASE "B" 48" x 21" HC SLAB



OTTER BROOK LAKE ENTRANCE BRIDGE, KEENE, N.H. FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 18 August 1993

<u>DATE OF PREVIOUS INSPECTIONS</u>: In-depth, *.

Routine, Sept 87 Routine, Sept 89

Routine, 22 August 1991

RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	18.0T	32.6T 39.9T	Load rating were recalculated for
3 3S2	22.1T 34.4T	62.1T	prestressed beams.

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway, Railings, and Deck.

The overall condition is good. (condition) 7

The bituminous wearing surface on the deck is in good condition with minor rutting along the wheel lines. There is some minor rutting at the gravel approach on the south side. The bituminous concrete road on the north side has a four inch pothole and some minor rutting along wheel lines. The pavement is also cracked along the slab transition due to one-half inch settlement. The approach guardrails are in very good condition with the exception of a slightly bent end rail on the southeast corner.

- B. Fascia and Curbs
- The overall condition is good (condition 7). Both the curbs and fascias have hairline cracks approximately every two feet. Minor debris and vegetation along the curbing.
- C. Underside of Deck and Bearings.
- The overall condition is good (condition 7). The underside of the deck is in good condition. No problems were observed with the bearings.
- D. Wingwalls and Abutments
- The overall condition is fair (condition 6). In general, the cementitious coating is delaminating and in poor condition.

The abutments appear to be stable. Spalling has occurred on the southeastern wingwall. On the north abutment there is an eight foot by two foot by six inch spall on the northeast corner and a four foot by two foot by six inch spall on the northwest corner. There is also evidence of scour and undermining on the north abutment.

E. Channel

Scour is occurring from four foot deep to the top of the water line on the north abutment with some undermining taking place.

CONDITION RATING

Interim 1987: 7 Interim 1989: 6 Routine 1991: 7 Routine 1993: 7

RECOMMENDATIONS

Status of Previous Recommendations

	Status of Previous Reco	illiendacions	
		Cost Est	Status
1.	Repair erosion and deteriorated concrete at the base of the abutments and wingwalls.	\$15,000	Not Done
2.	Replace bituminous pavement at north approach.	\$3,500	Not Done
3.	Remove all deteriorated concrete repair mortar in wingwalls and abutments and replace with new to give uniform surface.	\$12,500	Not Done
4.	Replace nuts on railing post cap.	Maint.	Not Done
5.	Remove vegetation from wingwalls and curbs.	\$1000	Not Done
	Total	al \$32,000	

Revised Recommendations

Implement above recommendations.

Total Updated Estimated Cost \$32,000

STRUCTURES INSPECTION FIELD REPORT

ROUTINE INSPECTION

city/	KEENE, I			bridge dep		8-structur	re no.	90-date inspected 8 18 93
2-dist.	104-highway sys		22-owner	LWE	27-year b		106-year rebuilt 1987	11-milepoint
43-structure		CONCRETE B	eldge B	E4MS_	quality co	NICK		
07-facility ca	rried EC AREA	ENTLANCE			team lea		دهسد ،	
06-features i		RMIRANCE			team me			
01	TER BROW) <u>k</u>				EUEN	I lorio MARK	. DESCHENES
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PROJECT: OFTER BROOK

NAME: ENTRAUCE BRIDGE

LOCATION: KEENE, NH

BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	YES
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	YES
a. Piers, abutments with spread footings or short pile foundations.	N/A
 b. Superstructure with simple spans or non- redundant support systems. 	YES
c. Inadequate waterway openings.d. Designs which collect ice and debris.	NO YES
e. All water must pass through or over structure.	720
f. Other.	NA
3. Are any characteristics of an aggressive stream or waterway present?	YES
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of 	YES
streambanks.	NO
c. Steep slopes.	10
d. High velocities. e. Any history of highway or bridge damage during past floods.	<u>00</u>
f. Other.	NIA
4. Is the bridge located on a stream reach with any adverse flow characteristics?	No
a. Crossing near stream confluence.b. Crossing of tributary stream near confluence	No
with larger streams.	No
c. Crossing on sharp bend in stream.	No
d. Location on alluvial fan.e. Other.	NO N/K
	. 1 4-
5. Other comments or observations.	<u>NA</u>

NEW ENGLAND DIVISION

CORPS OF ENGINEERS, U.S. ARMY

	4.1
PAGE	

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NIBJECT	Extin 6	r 02-	ette fe.	EPANG	ENTRANCE	BRIDGE.

SUBJECT FATING FOR STOPE EPANE ENTERING BELLEG

INVENTORY	LOND PATINGS					
	·	HIS I	B	352		
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	SHERR	49.3	58.5	106.5		
48=21	FLEXULE	18.0	22.1	344		
	546+2	37.8	44.9	81.5		
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36 K21	FLEWORE	45.5	55.8	86.9		
	SEAR	82	97.6	177.5		
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	2000AQ	43.0	74.6	134.6		

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OVERALL PATING	HIS	3	352	
INVENTORY	18.0	22.1	34,4	
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NOTE: FOR CALCULATIONS, SEE EXIT BRIDGE

COLEBROOK LAKE BRIDGE ON OLD ROUTE 8 SANDISFIELD, MA FY 93 ROUTINE INSPECTION REPORT

DATE OF INSPECTION: 25 August 1993

<u>DATE OF PREVIOUS INSPECTIONS</u>: In-depth, Dec 84

Routine, Sept 87 Routine, Sept 89 Routine, June 91

RATING (T=TONS)

Type	Inventory	Operating	Comments
H15	24T	33T	
3	34T	52T	
3S2	52T	82T	
3-3	60T	98T	

EVALUATION (See attached "Structures Inspection Field Report")

A. Roadway, & Railings

The bridge deck, approaches, guardrails, and railings are in very good condition (condition 8). The new deck surface and approaches are still in good condition. There are no visible joints at either end of the bridge. Some of the aggregate from the chip seal surfacing has accumulated along the gutters on the bridge. The cable guardrails along the north approach are both loose. The guardrails along the bridge deck have recently been painted as part of the contract to paint the bridge.

B. Superstructure

The trusses and bracing are in good condition. The entire superstructure has been recently painted. The paint is in good condition, however, the contractor was limited to the amount of scraping that was allowed due to the use of lead in previous coats of paint. This may tend to lead to accelerated degradation of the new finish. All joints, welds, and connections are in good condition. Most deteriorated rivets have been replaced with high strength bolts.

C. Underside of Deck

The superstructure under the deck is in good condition. The floor beam connections at the bearings at the ends of the trusses on the inside of the skew angle are filled with sand and painted. Attention will have to be paid to this area in future inspections since it is a likely spot for corrosion. Otherwise they should be cleaned out, filled with concrete, and capped. There is some honeycombing along the underside of the deck. Some remaining burlap was noticed between the floor beams and stringers. The bearings are in good condition.

D. Wingwalls and Abutments

The wingwalls and abutments are in good condition. Most vertical cracking has been sealed as recommended in previous inspections. Some horizontal cracking along cold joints in both wingwalls and abutments have not been repaired. Some minor efflorescence was noted along both north and south wingwalls.

E. Channel

The channel is in good condition and flowing smoothly. There is a moderate amount of rubble built up in the north side of the channel. The sheetpile and concrete toe protection along the south abutment is in good condition.

CONDITION RATING

In-depth 7
Interim 1987 7
Interim 1989 6
Routine 1991 7
Routine 1993 8

RECOMMENDATIONS

Status of Previous Inspections

Item

- 1. Remove vegetation from southeast wingwall
- 2. Remove vegetation from curb edge.
- 3. Paint structural steel

Not Done Recurring Complete

Status

Revised Recommendations

Keep the curb edge free of vegetation. No additional recommendations

STRUCTURES INSPECTION FIELD REPORT

ROUTINE INSPECTION

bridge o					i	8-structu	4	90-date inspected
		SOSFIE						25 AUG 93
2-dist.	104-highway system		22-owner		27-year t	ouilt	106-year rebuilt	11-milepoint
2-dist. 43-structure 07-facility ca 06-features item 58 DECK 1. Wea 2. Dec 3. Stay 4. Curl 5. Mec 6. Side 7. Par 8. Rail 9. Anti 10. Dra 11. Ligh	type STEL arried DEDUTE Sintersected Arris Tan aring Surface ck-Condition in Place Forms bs dian ewalks apet ling Missile Fence ins atting Standards	euss	item 59 SUPERST 1. Bearing 2. Stringe 3. Diaphra 4. Girders 5. Floor B 6. Trusse 7. Rivets 8. Welds 9. Collision 10. Load D	agms s or Beams seams or Bolts on Damage Deflection er Alignment	team lea team m	ouilt ontrol eng CIC 3 ider SEDH embers	106-year rebuilt	In-milepoint M. 10R10 In The seats
12. Utili 13. Dec 14. App	-		14. Year P	ainted Clearance _	ft _	in ; ∑ no	f-Scour g-Settlem 3. Collision Damag 4. Hydraulic-Adequ	e 🖃
					Overhe	ad Signs ((attached to bridge)	
Actual Pos	nded Posting	3 3S2	Single			yes	hy uo	
From Ratin					 Weld Boits 	s _ ,	-	
SIGNS IN Y or N	PLACE at bridg	je	advand [V]	ce .	3. Cond	lition	_	
LEGIBILI	ry 🖃				Item93b	U/W Ir	nspection Date:	¥:
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X=UN	KNOWN		NA-NOT	ADDLIO	ADIE		IA-INIACCI	TOOIDLE :

PROJECT: COLE PROVE LAKE

NAME: OLD POITE & FAITH

LOCATION: SANDING FIELD LIN

BRIDGE INSPECTION SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	NO
2. Is the streambed erodible? If so, does the structure have any vulnerable design features?	Yes
 a. Piers, abutments with spread footings or short pile foundations. b. Superstructure with simple spans or non-redundant support systems. c. Inadequate waterway openings. d. Designs which collect ice and debris. e. All water must pass through or over structure. 	NO NO
f. Other. 3. Are any characteristics of an aggressive stream or waterway present?	<u> </u>
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	20 20 20 20 40
4. Is the bridge located on a stream reach with any adverse flow characteristics?	N.
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	17.0 13.0 13.1
5. Other comments or observations.	AND THE PERSON NAMED IN

KNIGHTVILLE DAM INDIAN HOLLOW ROAD BRIDGE, HUNTINGTON, MA FISCAL YEAR 1993 ROUTINE INSPECTION REPORT

DATE OF ROUTINE INSPECTION: 25 August 93

DATE OF PREVIOUS INSPECTIONS: Routine Inspection, 13 May 91 Inventory Inspection, March 85

RATING (T = TONS)

Type	Inventory	Operating	Comments
H	17T	40T	No change in ratings due to inspection findings.
Type 3	20T	47T	
Type 3S2	29T	67T	

EVALUATION (See attached "Structures Inspection Field Report")

		-
Α.	Superstructure -Above Deck	 -Overall condition is good. -Both east and west approaches are in fair to good condition. The bituminous pavement at the west approach is unravelling. -There are no bridge railings or approach guardrails. -The wearing surface on the deck is in good condition, with a small amount of sand debris collecting at the curbs.
В.	Superstructure -Below Deck	-Overall condition is goodThe underside of the prestressed concrete planks is in good condition. There are signs of water leakage between the planks near the west abutment.
c.	Substructure	-Overall condition is goodBoth east and west abutments are in good condition. Both have numerous hairline cracks with efflorescence, but this condition is not considered serious.
D.	Channel	-The channel under the bridge is in fair condition, with overgrowth of vegetation, but no signs of scour.
Ε.	Overall Numerical Condition Rating	Inventory 1985: 7 Routine 1991: 7

Routine 1993:

7

RECOMMENDATIONS

Status of Previous Recommendations

- Construct a 10' long by 12' wide bituminous approach slab at both approaches.
 A contract is currently underway.
- Construct 25' of approach guardrail at each of the four corners of the bridge.
 \$5,000 Not Done
- 3. Seal cracks in abutments.

 A contract is currently underway.

Revised Recommendations

1. Due to the low ADT on Indian Hollow Road, and the low vehicle speeds, it is not recommended to provide approach guardrails. There are no further recommendations at this time.

STRUCTURES INSPECTION FIELD REPORT

ROUTINE INSPECTION

city/l Huntington	M		bridge dep	t. no.	8-structu	ire no. DMA 2510020	90-date inspected 8/25/93
2-dist. 104-highway system	7774	22-owner Corps of	Eng.	27-year l	puilt	106-year rebuilt	11-milepoint
43-structure type Prestressed Conc. Slab 07-facility carried Indian Hollod 06-features intersected	, Simple S Roa		ury.		ontrol eng Mic ader Jos	k Forbes	
Little River	<u>, </u>		A Autoria Alba	Mike	Walst	, Mark Desch	ene s
item 58 DECK 1. Wearing Surface 2. Deck-Condition 3. Stay in Place Forms 4. Curbs 5. Median 6. Sidewalks 7. Parapet 8. Railing 9. Anti Missile Fence 10. Drains 11. Lighting Standards 12. Utilities 13. Deck Joints 14. Approach Settlement	7 8 7 N N N N N N	1. Bearing 2. Stringe 3. Diaphri 4. Girders 5. Floor E 6. Trusse 7. Rivets 8. Welds 9. Collision 10. Load E 11. Member 12. Load V 13. Paint-E 14. Year F	agms s or Beams s eams or Bolts on Damage Deflection er Alignmen //ibration Epoxy Painted Clearance		7 8 N N N N N in	item 60 SUBSTRUCTURE 1. Abutments	all Seats 7 wall 7 gs 7 nent 7 ment 7
Actual Posting H Recommended Posting	3 382	Single		Overhe 1. Weld	yes	(attached to bridge) no	
From Rating Book SIGNS IN PLACE at brid Y or N LEGIBILITY	20 29 dge	advan	ce	2. Bolts 3. Cond	dition	nspection Date: 3/25	. /93
annel scour 2. embankment erosion 3. fender system 4. spur dikes & jetties	8 5. rip r 3 6. effect 1 7. debt 1 8. vege	— [_/] ap or slope pa ctiveness ris	7 8 6	1. bri 2. tra 3. ap 4. gu	raffic Saf dge railir Insitions proach g ardrail te	juardrail <u>O</u>	condition

Knightville Dam Indian Hollow Bridge

SCOUR CHECKLIST

1. Is the bridge currently experiencing, or does it have a history of, scour activity?	_ No
2. Is streambed <u>erodible?</u> If so, does the structure have any vulnerable design features?	Yes_
a. Piers, <u>abutments</u> with <u>spread footings</u> or short pile foundations. b. Superstructures with <u>simple spans</u> or non-redundant support systems. c. Inadequate waterway opening. d. Designs which collect ice and debris. e. All water must pass through or over structure. f. Other.	/es
3. Are any characteristics of an aggressive stream or waterway present?	
 a. Active degradation or aggredation of streambed. b. Significant lateral movement or erosion of streambanks. c. Steep slopes. d. High velocities. e. Any history of highway or bridge damage during past floods. f. Other. 	
4. Is bridge located on stream reach with any adverse flow characteristics?	
 a. Crossing near stream confluence. b. Crossing of tributary stream near confluence with larger streams. c. Crossing on sharp bend in stream. d. Location on alluvial fan. e. Other. 	
5. Other comments or observations.	

Appendix A

Visual Assessment for Scour Potential

Everett Lake ---- Choate Brook
Birch Hill ----- Goodnow Road
Old Route 202
Middle Road

GEOTECHNICAL ASSESSMENT

FOR

BRIDGE SCOUR STUDY

AΤ

CHOATE BROOK BRIDGE

EVERETT RESERVOIR

WEARE, NEW HAMPSHIRE

GEOTECHNICAL ASSESSMENT

FOR

BRIDGE SCOUR STUDY

AΤ

CHOATE BROOK BRIDGE

EVERETT RESERVOIR

WEARE, NEW HAMPSHIRE

TABLE OF CONTENTS

		Page No.
I.	INTRODUCTION 1.1 General	1
	1.2 Purpose and Scope	
II	SITE CONDITIONS 2.1 Site Location and Description 2.2 Bridge Description 2.3 Site Geology	2
	2.4 Streambed and Streambank	
II	I. ASSESSMENT	4
	3.1 Streambed Material Characteristics3.2 Streambank Material Characteristics3.3 Scour Potential	
	3.4 Proposed Remedial Work at Choate Brook Bridge	
IV	. APPENDIX Locus Plan	6
	Choate Brook Bridge Sketch	

I. INTRODUCTION

1.1 General

This report presents a visual assessment of scour potential at Choate Brook Bridge which is situated in the reservoir area of Everett Dam. The work was done by Geotechnical Engineering Division as part of the NED Bridge inspection program.

1.2 Purpose and Scope

The purpose of the assessment was to obtain information on subsurface and streambed conditions at Choate Brook Bridge and visually evaluate whether there is a potential for scour around the footings and abutments. The scope of work included:

- a. Field reconnaissance of the site during September 1993.
- b. Research of available geological and geotechnical information.
- c. Laboratory testing of streambed samples collected during the September 1993 field reconnaissance of the site.
- d. Report to include locus plan, gradation curve, site description, subsurface and streambed conditions, and assessment.

II. SITE CONDITIONS

2.1 Site Location and Description

Everett Dam and reservoir are located along the Piscataquog River, a tributary of the Merrimack River, in south central New Hampshire. Choate Brook is a tributary of the southeasterly flowing Piscataquog River, as shown on the Locus Plan in the Appendix. The bridge is in the northern portion of Everett Lake (recreational pool level) and within one-quarter mile of the normal Piscataquog River channel. Choate Brook has a fairly flat slope in the vicinity of the bridge. It cuts through a relatively flat floodplain. A moderate sloping hill ascends to the west of the bridge. A rough sketch (plan view) of the bridge and adjacent areas is included in the Appendix.

2.2 Bridge Description

Choate Brook Bridge has a concrete slab deck which bears on rubble masonry abutments and footings. A smooth concrete surface has been cast against the west abutment. The abutments and footings appear to be in fair to good condition. Stone revetments protect the corners of the bridge. The outer layer of the revetments are in good condition. However, there does not appear to be filter layers between the outer layer and the subgrade.

The footings of the bridge are founded on sand and gravel. It appears high water velocities have eroded (scoured) the sand and gravel below the south end of the west abutment footing. The void is approximately five feet wide by two feet high and is up to two feet deep. Distress cracks were not noted in the abutment area above the void.

Recently several small repairs have been made to the footings, revetments, and abutments. An apparent void under the north end of the west abutment footing was filled with concrete. Voids between the stones in the top two feet of the east abutment were filled with grout. Voids in the stone revetments at the north end of the bridge were filled with grout. Generally the work looks good except that an area up to three feet wide was not grouted at the junction of the stone revetment and northeast corner of the bridge.

2.3 Site Geology

Choate Brook flows through a low, flat and relatively wide area in the pre-glacial Piscataquog River valley. The valley has been filled with deep glacial outwash deposits and till. The brook has eroded a narrow valley in the outwash deposits and the till. Till and till covered bedrock hills which rise above the lowlands form the perimeter of the brook's drainage area.

2.4 Streambeds and Streambanks

The streambed is slightly meandering. It consists of clean, fine to coarse, sands and gravels with rounded to subangular cobbles and boulders. Gradations for the matrix portion of the streambed are included in the appendix. The cobbles and boulders in the streambed are typically 0.25 to 0.75 feet diameter with a maximum diameter of 1 foot. A beaver dam were observed at the north end of the bridge in the streambed. Water flowed through the dam rather than over the top during the inspection. It is approximately five feet high and 12 feet wide at the base. The water level was approximately 4.5 feet deep upstream of the dam and 2.5 feet deep downstream.

The streambanks are typically fairly low (ten feet or less high) and flat (1 vertical on 3 horizontal to 1 vertical on 10 horizontal). Due to the width of the channel in the vicinity of the bridge, the slopes are not critical. Medium to dense

vegetation grows on the banks.

III. ASSESSMENT

3.1 Streambed and Streambank Material Characteristics

The streambed materials are deep deposits of hard, durable, rounded to subangular, sands, gravels, cobbles and boulders. The mean diameter, by weight, of the sand to boulder sized materials was visually estimated to be 0.25 to 0.5 feet at Choate Brook Bridge. Laboratory gradation tests (Complete gradation test results are in Appendix.) were performed on samples of the sand and gravel matrix materials that exist between the cobbles and boulders. The results indicate that the mean diameter, by weight, of the streambed matrix materials sampled is 1.5 millimeters (0.06 inches). The mean diameters of the streambed materials could be used in theoretical hydraulic studies to estimate the scour potential around the abutment footings.

3.2 Streambank Materials Characteristics

The streambank matrix material characteristics did not appear to be significantly different than the streambed matrix materials. However, the number and sizes of cobbles and boulders in the streambank materials appeared to be lower than in the streambed materials.

3.3 Scour Potential

High water velocities have scoured the material below the footings at the bridge as described in paragraph 2.2. It appears that high water velocities that occur during future flood events will continue to erode the foundation and the bottom of footing materials. Continued erosion will reduce the bearing capacity of the footings and cause subsequent damage to the superstructure of the bridges at a faster rate than normal weathering.

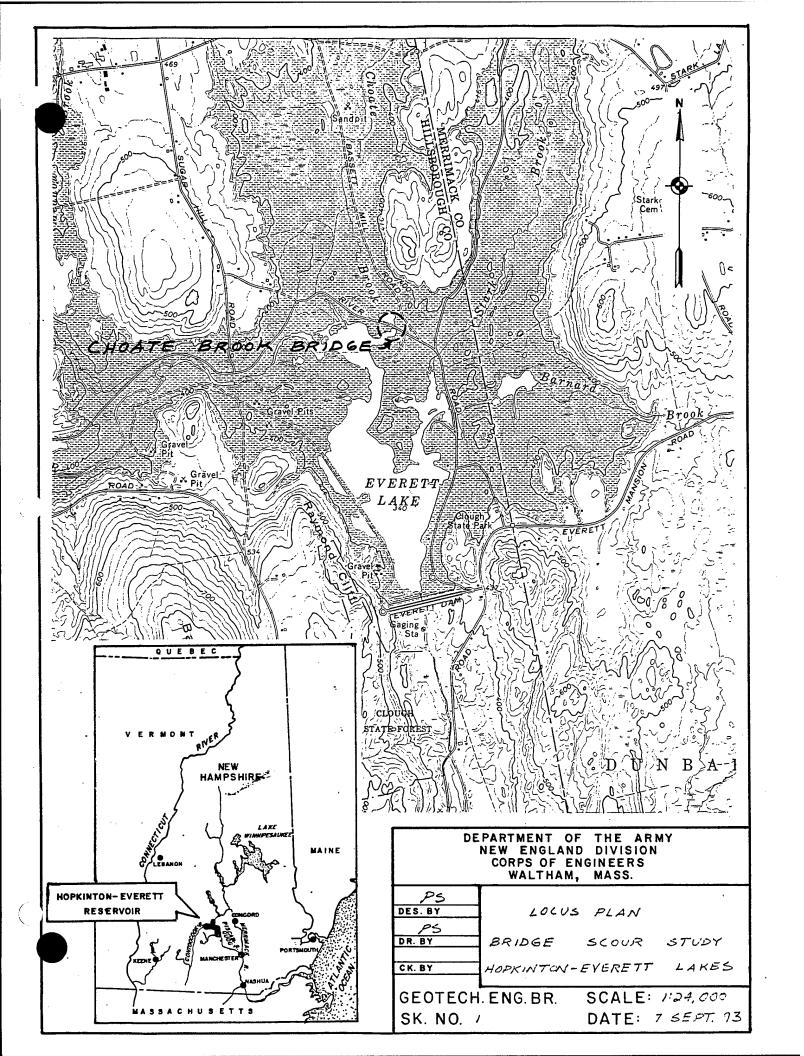
3.4 Proposed Remedial Work at Choate Brook Bridge

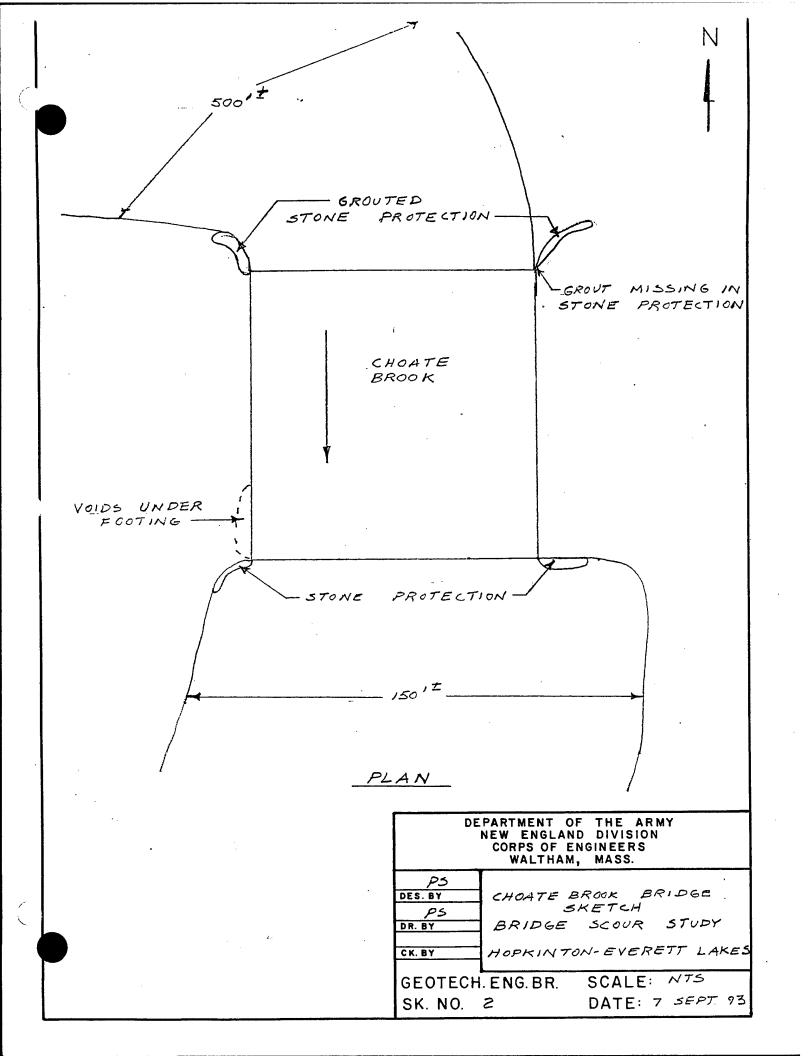
The most pressing need is to fill the void below the west abutment footing. It appears further erosion could damage the abutment. A possible method for repairing the footings is to place concrete forms around the outside edges and then pump concrete into the eroded voids and the space between the footings and forms. Then the entire channel (from approximately 15 feet upstream to 15 feet downstream of the bridge) should be lined with a stone blanket (estimated thickness of 2 feet) underlain by a bedding layer (estimated thickness of 1 foot). The stone blanket and bedding should extend to the top of the banks upstream and downstream of the bridge.

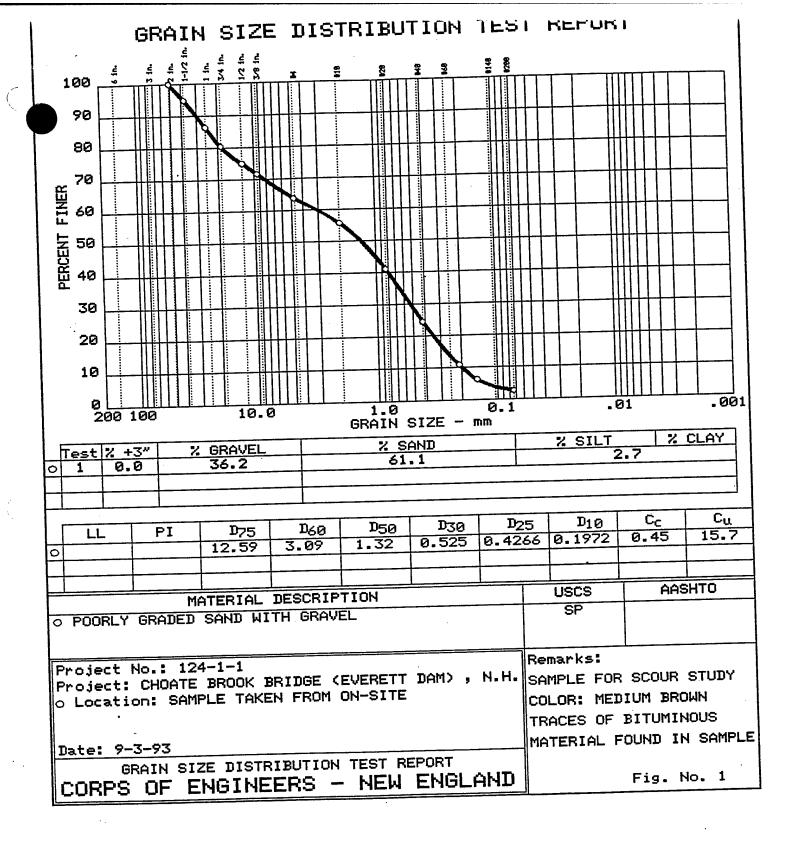
Grout should be placed in the voids of the stone revetment at the junction of the stone revetment and the northeast corner of the bridge. The junction is area of potential future scour because it is weaker than the bridge abutment and grouted stone revetment on either side of it. Approximately one cubic yard of grout and a few hours of hand labor would be required to place the grout.

It is recommended that the beaver dam that was observed upstream of Choate Brook Bridge be removed. It appears that the dam might slightly alter the hydraulic characteristics of the stream and cause eddy currents which could lead to additional scour near the bridge. Rental of a small truck and a few hours of hand labor would be needed to remove the debris.

IV. APPENDIX







GEOTECHNICAL ASSESSMENT

FOR

BRIDGE SCOUR STUDY

AΤ

THREE BRIDGES

BIRCH HILL RESERVOIR

WINCHENDON, MASSACHUSETTS

GEOTECHNICAL ASSESSMENT

FOR

BRIDGE SCOUR STUDY

AΤ

THREE BRIDGES

BIRCH HILL RESERVOIR

WINCHENDON, MASSACHUSETTS

TABLE OF CONTENTS

	Page No.
I. INTRODUCTION 1.1 General 1.2 Purpose and Scope	1
II. SITE CONDITIONS 2.1 Site Location and Description 2.2 Bridge Descriptions 2.3 Site Geology 2.4 Streambeds and Streambanks	2
III. ASSESSMENT 3.1 Streambed Material Characteristics 3.2 Streambank Material Characteristics 3.3 Scour Potential 3.4 Remedial and Maintenance Work at Goodnow Road Brid 3.5 Maintenance Work at Middle Road Bridge 3.6 Old Route 202 Bridge	ge
IV. APPENDIX Locus Plan Goodnow Road Bridge Sketch Middle Road Bridge Sketch Old Route 202 Bridge Sketch Gradation Curves	6

I. INTRODUCTION

1.1 General

This report presents a visual assessment of scour potential at three bridges situated in the reservoir area of Birch Hill dam. The work was done by Geotechnical Engineering Division as part of the NED Bridge inspection program.

1.2 Purpose and Scope

The purpose of the assessment was to obtain information on subsurface and streambed conditions at the three bridges and visually evaluate whether there is a potential for scour around their footings and abutments. The scope of work included:

- a. Field reconnaissances of the sites July 1993.
- b. Research of available geological and geotechnical information.
- c. Laboratory testing of streambed samples collected during a July 1993 field reconnaissance of the sites.
- d. Report to include locus plan, gradation curves, site description, subsurface and streambed conditions, and assessment.

II. SITE CONDITIONS

2.1 Site Location and Description

Birch Hill dam and reservoir are located along the Millers River, a tributary of the Connecticut River, in central Massachusetts. Priest Brook and Beaver Brook are tributaries of the southerly flowing Millers River, as shown on the Locus Plan in the Appendix. The Goodnow Road and Middle Road bridges cross Priest Brook. The Old Route 202 bridge crosses Beaver Brook. The three bridges are within one-half mile of the normal Millers River channel. The tributaries have fairly flat slopes in the vicinity of the three bridges and cut through a relatively flat floodplain. A moderate sloping hill ascends to the north of Old Route 202 bridge. Sketches (plan views) of the bridges and adjacent areas are included in the Appendix.

2.2 Bridge Descriptions

The three bridges have steel girder and concrete decks which bear on concrete abutments and footings except for the Middle Road bridge where the deck bears on chinked stone and mortar abutments and footings. Concrete wingwalls (at each corner) protect Goodnow Road and Old Route 202 bridges while chinked stone and mortar wingwalls (at each corner) protect Middle Road bridge. Gabion extensions have been added to the concrete wingwalls at Goodnow Road bridge.

It appears that the footings for all the bridges are founded on sand and gravel. The footings are in good condition except for the ones at Goodnow Road bridge which have been undermined. It appears high water velocities have eroded (scoured) the sand and gravel below the concrete footings at Goodnow bridge. A steel bar could be pushed from 0.5 to 3.5 feet into nine voids under the south abutment footing and 0.5 to 1 feet into six voids under the north abutment footing. Although voids were observed under the footings at Goodnow Road bridge, no distress cracks other than normal weathering were noted in the abutments.

2.3 Site Geology

The Millers River flows through a wide pre-glacial bedrock valley in the vicinity of the three bridges. The valley has been filled with deep glacial outwash deposits of sands and gravels. The river has eroded a narrow inner valley in the sands and gravels which is flanked by sand and gravel terraces. Priest and Beaver Brooks are tributaries that have cut narrow channels through the terraces to the river.

2.4 Streambeds and Streambanks

The streambeds of the two tributaries are slightly meandering.

They consist of clean, fine to coarse, sands and gravels with rounded to subangular cobbles and boulders. Gradations for the matrix portion of the streambed are included in the appendix. The cobbles and boulders in the streambed are typically 1 to 3 feet in diameter with a maximum diameter of 8 feet at the Goodnow Road bridge, typically 0.5 to 1.5 feet in diameter with a maximum diameter of 2 feet at Middle Road bridge and typically 0.25 to 0.75 feet diameter with a maximum diameter of 1 foot at Old Route 202 bridge. Two large boulders (6 to 8 feet in diameter) were observed in the streambed under Goodnow Road bridge. Also a pile (10 by 20 feet) of branches was observed upstream of Goodnow Road bridge. Beaver dams were observed Under Middle Road bridge and approximately 100 feet upstream of the bridge. They were approximately two and three feet high respectively.

The streambanks are typically fairly low (five feet or less high) and steep (1 vertical on 1 horizontal to 1 vertical on 3 horizontal). Medium to dense vegetation grows on the banks. A small amount of erosion of the bank materials at Middle Road bridge was observed. It has occurred 5 to 10 feet upstream of the chinked stone wingwalls. The two eroded areas (scour holes) are 20 to 25 feet long and up to 7 feet wide. It does not appear that the

erosion is endangering the wingwalls or the bridge.

III. ASSESSMENT

3.1 Streambed and Streambank Material Characteristics

The streambed materials are deep deposits of hard, durable, rounded to subangular, sands, gravels, cobbles and boulders. The mean diameter, by weight, of the sand to boulder sized materials was visually estimated to be 1 to 1.5 feet at the Goodnow Road bridge, 0.5 to 1 feet at the Middle Road bridge and 0.25 to 0.5 feet at the Old Route 202 bridge. Laboratory gradation tests (Complete gradation test results are in Appendix.) were performed on samples of the sand and gravel matrix materials that exist between the cobbles and boulders. The results indicate that the mean diameter, by weight, of the streambed materials sampled is 1.5 millimeters (0.06 inches) at the Goodnow Road bridge, 0.63 millimeters (0.025 inches) at the Middle Road bridge and 10.1 millimeters (0.40 inches) at the Old Route 202 bridge. The mean diameters could be used in theoretical hydraulic studies to estimate the scour potential around the abutment footings.

3.2 Streambank Materials Characteristics

The streambank matrix material characteristics did not appear to be significantly different than the streambed matrix materials. However, the number and sizes of cobbles and boulders in the streambank materials appeared to be lower than the streambed materials.

3.3 Scour Potential

High water velocities have scoured material below the footings at Goodnow Road bridge as described in paragraph 2.2. It Appears high water velocities that have occurred during past flood events have not been a problem at Middle Road and Old Route 202 bridges. Field observations and measurements indicate that the top of the streambed is higher adjacent to the abutment footings than at the center of the stream channels at the Middle Road and Old Route 202 bridges. However, high water velocities have eroded streambank materials upstream of Middle Road bridge as described in paragraph 2.4. It appears the erosion of the streambanks near Middle Road bridge is due to the fact that there are fewer and smaller cobbles and boulders in the streambanks as noted in paragraph 3.2.

3.4 Remedial and Maintenance Work at Goodnow Road Bridge

The most pressing need is to fill the voids below Goodnow Road bridge footings. It appears further erosion could damage the abutments. A possible method for repairing the footings is to place concrete forms around the outside edges and then pump concrete into the eroded voids and the space between the footings and forms. Then the entire channel (from approximately 15 feet

upstream to 15 feet downstream of the bridge) should be lined with a stone blanket (estimated thickness of 2 to 3) underlain by a bedding layer (estimated thicknes of 1 to 1.5 feet). The stone blanket and bedding should extend to the top of the banks upstream and downstream of the bridge.

It recommended that the tree debris that was observed upstream of Goodnow bridge be removed. It appears that the debris might slightly alter the hydrologic characteristics of the stream and cause eddy currents which could lead to additional scour near the bridge. Rental of a small truck and a few hours of hand labor would be needed to remove the debris.

3.5 Maintenance Work at Middle Road Bridge

It recommended that the Beaver dam that was observed under Middle Road bridge be removed. It appears that the dam alters the hydrologic characteristics of the stream and causes eddy currents which could lead to scour near the bridge. Rental of a small truck and a few hours of hand labor would be needed to remove the dam.

It is recommended that the erosion upstream of Middle Road bridge be monitored during future bridge inspections. If it appears that the erosion is beginning to endanger the wingwalls, properly designed stone revetments should be constructed to reduce the erosion. They should extend from the streambed to the top of the streambank. The stone revetments are not expected to be major remedial items.

3.6 Old Route 202 Bridge

Substantial scour problems were not observed near the Old Route 202 bridge. No remedial or maintenance measures are recommended now at the Old Route 202 bridge.

IV. APPENDIX

